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BLACK AND VEATCH KANSAS CITY MO

NATIONAL DAM SAFETY PROGRAM. LAKE CYRENE DAM (MO 11596), MISSOURI-ETC(U)

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**MISSOURI-KANSAS CITY BASIN**

**AD A106509**

**LAKE CYRENE DAM**

**BOONE COUNTY, MISSOURI**

**MO 11596**

**PHASE 1 INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM**

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**PREPARED BY: U.S. ARMY ENGINEER DISTRICT. ST. LOUIS**

**FOR: STATE OF MISSOURI**

**MARCH 1981**

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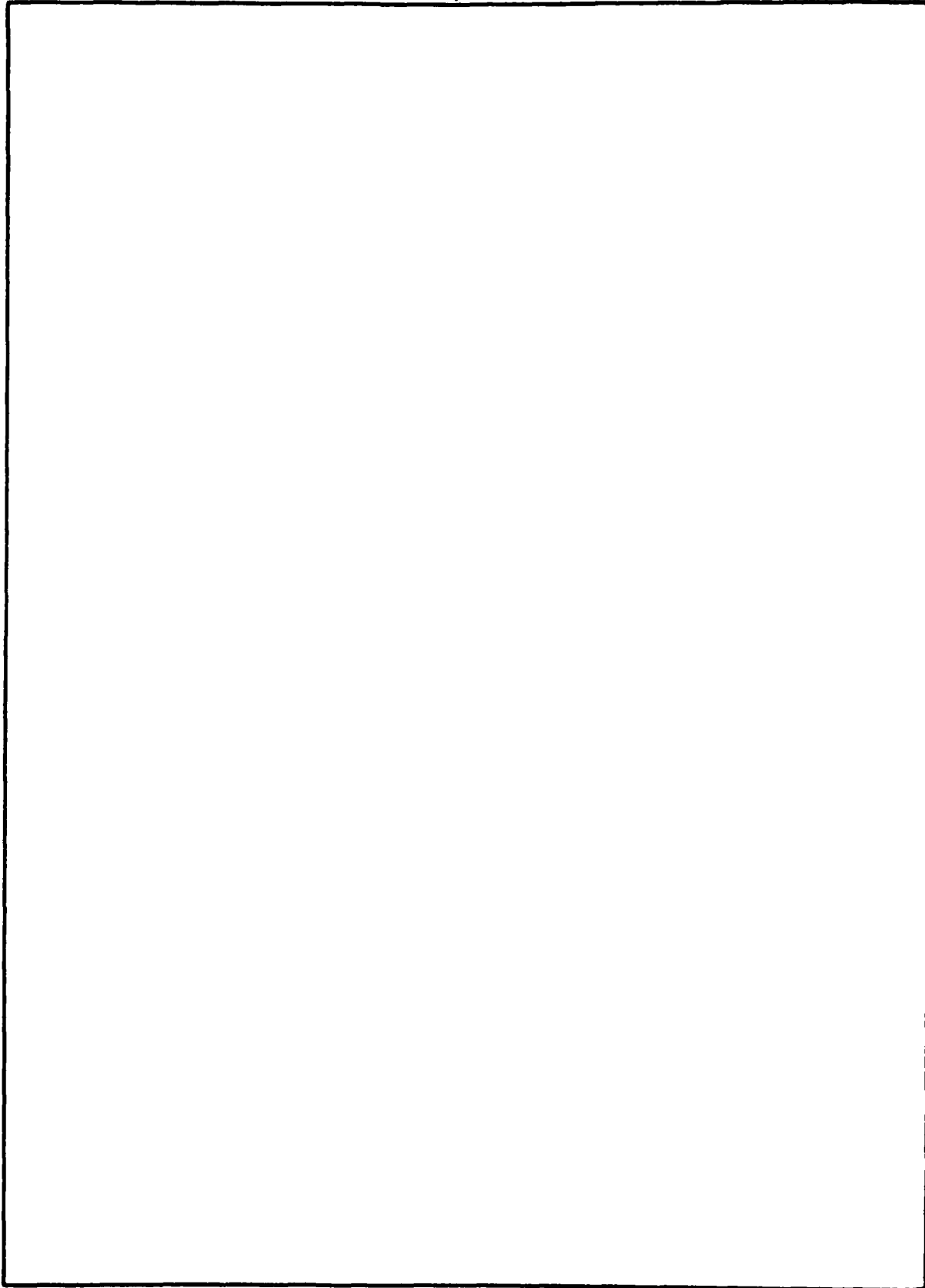
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# **MISSOURI-KANSAS CITY BASIN**

**LAKE CYRENE DAM  
BOONE COUNTY, MISSOURI  
MO 11596**

## **PHASE 1 INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM**



**United States Army  
Corps of Engineers**  
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**St. Louis District**

**PREPARED BY: U.S. ARMY ENGINEER DISTRICT. ST. LOUIS**

**FOR: STATE OF MISSOURI**

**MARCH 1981**



**DEPARTMENT OF THE ARMY**

**ST. LOUIS DISTRICT, CORPS OF ENGINEERS**

**210 TUCKER BOULEVARD NORTH**

**ST. LOUIS, MISSOURI 63101**

REPLY TO  
ATTENTION OF

SUBJECT: Lake Cyrene Dam, (MO 11596)  
Phase I Inspection Report

This report presents the results of field inspection and evaluation of the Lake Cyrene Dam (MO 11596).

It was prepared under the National Program of Inspection of Non-Federal Dams.

This dam has been classified as unsafe, non-emergency by the St. Louis District as a result of the application of the following criteria:

- a. Spillway will not pass 50 percent of the Probable Maximum Flood without overtopping the dam.
- b. Overtopping of the dam could result in failure of the dam.
- c. Dam failure significantly increases the hazard to loss of life downstream.

SUBMITTED BY:

**SIGNED**

Chief, Engineering Division

**22 JUL 1981**

Date

APPROVED BY:

**SIGNED**

Colonel, CE, Commanding

**23 JUL 1981**

Date

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LAKE CYRENE DAM  
BOONE COUNTY, MISSOURI  
MISSOURI INVENTORY NO. 11596

PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM

PREPARED BY:  
BLACK & VEATCH  
CONSULTING ENGINEERS  
KANSAS CITY, MISSOURI

UNDER DIRECTION OF  
ST. LOUIS DISTRICT CORPS OF ENGINEERS  
FOR  
GOVERNOR OF MISSOURI

MARCH 1981

PHASE I REPORT  
NATIONAL DAM SAFETY PROGRAM

Name of Dam	Lake Cyrene Dam
State Located	Missouri
County Located	Boone County
Stream	Tributary to Hinkson Creek
Date of Inspection	11 March 1981

Lake Cyrene Dam was inspected by a team of engineers from Black & Veatch, Consulting Engineers for the St. Louis District, Corps of Engineers. The purpose of the inspection was to make an assessment of the general condition of the dam with respect to safety, based upon available data and visual inspection, in order to determine if the dam poses hazards to human life or property.

The guidelines used in the assessment were furnished by the Department of the Army, Office of the Chief of Engineers and developed with the help of several Federal and state agencies, professional engineering organizations, and private engineers. Based on these guidelines, this dam is classified as a small size dam with a high downstream hazard potential. According to the St. Louis District, Corps of Engineers, failure would threaten lives and property. The estimated damage zone extends approximately two miles downstream of the dam. Within the estimated damage zone are four dwellings, one building, three apartment buildings, a gasoline station, a trailer court consisting of approximately 15 trailers, and the junction of Highways 740 and 63. Contents of the estimated downstream damage zone were verified by the inspection team.

Our inspection and evaluation indicates the spillways do not meet the criteria set forth in the guidelines for a dam having the above size and hazard potential. The spillways will not pass the probable maximum flood without overtopping but will pass 10 percent of the probable maximum flood. The spillways will not pass the flood which has a one percent chance of occurrence in any given year (100-year flood), but will pass the flood with a 10 percent chance of occurrence (10-year flood). The spillway design flood recommended by the guidelines is 50 to 100 percent of the probable maximum flood. Considering the hazard zone and the reservoir storage volume, the spillway design flood should be 100 percent of the probable maximum flood. The probable maximum flood is defined as the flood discharge which may be expected from the most severe combination of critical meteorologic and hydrologic conditions which are reasonably possible in the region.



Based on visual observations, this dam appears to be in less than satisfactory condition. Deficiencies visually observed by the inspection team were erosion and sloughing of the upstream face, seepage on the downstream face, trees growing on the embankment and animal burrows on the embankment. Seepage and stability analyses required by the guidelines were not available.

There were no observed deficiencies or conditions existing at the time of the inspection which indicated an immediate safety hazard. Future corrective action and regular maintenance will be required to correct or control the described deficiencies. In addition, detailed seepage and stability analyses of the existing dam, as required by the guidelines, should be performed. A detailed report discussing each of these deficiencies is attached.

Edwin R. Burton

Edwin R. Burton, PE  
Missouri E-10137

Harry L. Callahan

Harry L. Callahan, Partner  
Black & Veatch



OVERVIEW OF DAM

PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM  
LAKE CYRENE DAM

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## SECTION 1 - PROJECT INFORMATION

### 1.1 GENERAL

a. Authority. The National Dam Inspection Act, Public Law 92-367, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a program of safety inspection of dams throughout the United States. Pursuant to the above, the District Engineer of the St. Louis District, Corps of Engineers, directed that a safety inspection of the Lake Cyrene Dam be made.

b. Purpose of Inspection. The purpose of the inspection was to make an assessment of the general condition of the dam with respect to safety, based upon available data and visual inspection, in order to determine if the dam poses hazards to human life or property.

c. Evaluation Criteria. Criteria used to evaluate the dam were furnished by the Department of the Army, Office of the Chief of Engineers, in "Recommended Guidelines for Safety Inspection of Dams." These guidelines were developed with the help of several Federal agencies and many state agencies, professional engineering organizations, and private engineers.

### 1.2 DESCRIPTION OF PROJECT

#### a. Description of Dam and Appurtenances.

(1) The dam is an earth structure located in the valley of a tributary to Hinkson Creek. (See Plate 1). The watershed is an area of low hills with fairly steep slopes which is totally developed as a residential area. The dam is approximately 670 feet long along its crest. Its alignment forms a "Z". The embankment height is 25 feet from its crest to the toe of slope at its highest point. The crest width is about 10 feet. The downstream face of the dam has a nonuniform slope from the crest to the valley floor below.

(2) The principal spillway is a 36-inch corrugated metal pipe drop inlet connected to an 18-inch corrugated metal pipe through the dam. The pipe outlet is downstream of the dam and discharges into a small pond then to the natural stream below.

(3) The emergency spillway is a grass lined, open channel cut through the natural abutment at the left end of the dam. (Left or right as used herein is referenced while looking in a downstream direction.) The spillway channel is trapezoidal with a bottom width of about 7 feet. There is a low berm constructed along the right bank to prevent flow across the downstream slope of the embankment. The emergency spillway discharges onto a wooded hillside downstream of the dam.

(4) Pertinent physical data are given in paragraph 1.3.

b. Location. The dam is located in the eastern part of the City of Columbia in central Boone County, Missouri, as indicated on Plate 1. The lake formed by the dam is shown on the United States Geological Survey 7.5 minute series quadrangle map for Columbia, Missouri in Section 19 of T48N, R12W.

c. Size Classification. Criteria for determining the size classification of dams and impoundments are presented in the guidelines referenced in paragraph 1.1c above. Based on these criteria, the dam and impoundment are in the small size category. A small size dam is classified as having a height less than 40 feet, but greater than or equal to 25 feet and/or a storage capacity less than 1,000 acre-feet, but greater than or equal to 50 acre-feet. Lake Cyrene Dam is 25-feet high with a normal storage volume of 50 acre-feet.

d. Hazard Classification. The hazard classification assigned by the Corps of Engineers for this dam is as follows: The Lake Cyrene Dam has a high hazard potential, meaning that the dam is located where failure may cause loss of life, and serious damage to homes, agricultural, industrial and commercial facilities, and to important public utilities, main highways, or railroads. For the Lake Cyrene Dam the estimated flood damage zone extends approximately two miles downstream of the dam. Within the estimated damage zone are four dwellings, one building, three apartment buildings, a gasoline station, a trailer court with approximately 15 trailers, and the junction of Highways 740 and 63. Contents of the estimated downstream damage zone were verified by the inspection team.

e. Ownership. The dam is owned by the Lake Cyrene Corporation, 1132 Business Highway 63 South, Columbia, Mo. 65201, c/o William Platt, President.

f. Purpose of Dam. The dam forms a 6.3-acre lake used for recreation.

g. Design and Construction History. Data relating to the design and construction were not available. According to Mr. William Platt, president of the Lake Cyrene Corporation, the dam was built in the 1930's and provided water for dairy farming operations.

h. Normal Operating Procedure. Normal rainfall, runoff, transpiration, evaporation, and overflow through the uncontrolled spillway all combine to maintain a relatively stable water surface elevation.

### 1.3 PERTINENT DATA

a. Drainage Area - 64 acres

b. Discharge at Damsite.

(1) Normal discharge at the damsite overflows to a 36-inch diameter corrugated metal pipe drop inlet to an 18-inch diameter corrugated metal pipe.

(2) Estimated experienced maximum flood at damsite - Unknown.

(3) Estimated ungated spillway capacity at maximum pool elevation 110 cfs (Probable Maximum Flood Pool El. 686.5).

c. Elevation (Feet above m.s.l. Approximate Tie to USGS Map).

(1) Top of dam - 685.5 (see Plates 3 & 4)

(2) Principal spillway crest - 684.0

(3) Streambed at toe of dam - 660.3

(4) Maximum tailwater - Unknown.

d. Reservoir.

(1) Length of maximum pool - 700 feet  $\pm$  (Probable maximum flood pool level)

(2) Length of normal pool - 625 feet  $\pm$  (Principal spillway crest)

e. Storage (Acre-feet).

(1) Top of dam - 60

(2) Principal spillway crest - 50

(3) Design surcharge - Not available.

f. Reservoir Surface (Acres).

(1) Top of dam - 6.8

(2) Principal spillway crest - 6.3

g. Dam.

(1) Type - Earth embankment



- (2) Length - 670 feet
- (3) Height - 25 feet  $\pm$
- (4) Top width - 10 feet
- (5) Side slopes - upstream face 1.0 V on 2.3 H, downstream face between 1.0 V on 2.5 H and 1.0 V on 6.7 H (see Plate 4)
- (6) Zoning - Unknown.
- (7) Impervious core - Unknown.
- (8) Cutoff - Unknown.
- (9) Grout curtain - Unknown.
- h. Diversion and Regulating Tunnel - None.
- i. Principal Spillway.
  - (1) Type - 36-inch diameter CMP drop inlet with 18-inch diameter CMP through the dam.
  - (2) Inlet crest elevation - 684.0 feet m.s.l.
  - (3) Inlet invert elevation - 677.7.
  - (4) Outlet invert elevation - 669.8 feet m.s.l.
  - (5) Gates - None.
  - (6) Upstream channel - None.
  - (7) Downstream channel - Spillway discharges to a small pond downstream of the dam then to the natural stream below.
- j. Emergency Spillway.
  - (1) Type - Grass lined open channel.
  - (2) Crest Elevation - 684.6 feet m.s.l.
  - (3) Gates - None.
  - (4) Upstream Channel - Grass-lined approach channel.

(5) Downstream Channel - Spillway, discharges to wooded hillside then to a small pond downstream of the dam then to stream below.

k. Regulating Outlets - None.

## SECTION 2 - ENGINEERING DATA

### 2.1 DESIGN

Design data were not available.

### 2.2 CONSTRUCTION

Construction records were unavailable.

### 2.3 OPERATION

Operational records and documentation of past floods were unavailable.

### 2.4 GEOLOGY

The site of the dam and reservoir is located across a broad, moderately steep sided valley. The dam impounds the drainage from an intermittent tributary of Hinkson Creek.

The soil in the dam and reservoir area consists of Union silt loam and silty clay loam. The Union series consists of moderately well drained soils formed in loess over cherty residuum weathered from limestone. The upper 8 inches of this series is classified for engineering purposes as clayey silt to silty clay (Unified Classification CL-ML to CL). The remaining part of the soil profile is classified as silty clay (CL) with clayey sand (SC), clayey gravel (GC) and highly plastic clay (CH) present below a depth of 40 inches. Bedrock of the area consists of Mississippian age cherty, crinoidal limestones of the Osagean Series. Depth to bedrock could not be determined visually at the site but is assumed to be greater than 5 feet based on Soil Conservation Service information.

### 2.5 EVALUATION

a. Availability. No engineering data were available.

b. Adequacy. No engineering data were available. Thus, an assessment of the design, construction, and operation could not be made. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency. These seepage and stability analyses should be performed for appropriate loading conditions (including earthquake loads) and made a matter of record.

c. Validity. The validity of the design, construction, and operation could not be determined due to the lack of engineering data.

### SECTION 3 - VISUAL INSPECTION

#### 3.1 FINDINGS

a. General. A visual inspection of Lake Cyrene Dam was made on 11 March 1981. The inspection team consisted of Edwin Burton, team leader; Shannon Casey, geologist; Gary Van Riessen, geotechnical engineer; and John Ruhl, hydraulic/hydrologic engineer. The dam appeared to be sound but is in less than satisfactory condition due to erosion of the upstream face and trees and animal burrows on the embankment. Specific observations are discussed below. No observations were made of the condition of the upstream face of the dam below the pool elevation at the time of the inspection.

b. Dam. The inspection team observed the following conditions at the dam. There was no cracking or evidence of sliding of the embankment slopes. The dam crest elevation was very irregular but there was no visible sign that the dam had ever been overtopped. Mr. Platt, who has lived in the residence at the right end of the dam for over ten years, stated that he has no knowledge of the dam ever being overtopped.

The water line along the upstream face and the slope of the upstream face are very irregular due to wave action erosion and sloughing, a condition that is aggravated by burrowing animals, (muskrat). The upstream face has riprap slope protection; which is located at an elevation lower than the principal spillway inlet and the erosion and sloughing is taking place above the riprap, (Photo 15). Several large animal burrows (Photo 14), probably groundhog, were observed on the downstream face of the dam. An area of nonflowing seepage was observed on the downstream slope (Photo 7) and below the downstream toe near and to the right of an abandoned pumphouse. The seepage was evidenced by the abundant growth of cattails and aquatic plants. Another possible seepage area exists on the right abutment downstream of the dam. Mr. Platt indicated that this area stays wet most of the time. This area did not appear to be unusually wet to the inspection team; however, the lake level was low on the day of inspection. There are many trees growing on both faces of the embankment, (Photos 6, 7 and 15) some with trunks as large as 12 to 14 inches. A row of poplar trees 2 to 4 inches have been planted along the downstream edge of the crest. Erosion protection on the crest and downstream face consist of a good grass cover with some weeds.

c. Appurtenant Structures. Appurtenant structures observed by the inspection team include the principal spillway, the emergency spillway, and the old pump house.

The principal spillway is a 36-inch corrugated metal pipe drop inlet to a 18-inch corrugated metal pipe through the embankment. The

drop inlet has a welded steel bar trash screen at the inlet crest, (Photo 8). Only the inlet and about two feet of the pipe at the outlet end were observable. The pipe alignment was observed by looking through the pipe from the outlet end. The pipe appeared to turn up slightly at about 30 feet from the outlet. There appeared to be a slight crimp at the crown of the pipe at the point where it turned up. The pipe and drop inlet were of sound metal with some surface rust. There was no flow through the pipe at the time of the inspection.

The emergency spillway is an open channel cut through natural material of the left abutment. No signs of erosion of the emergency spillway were observed. The channel floor and banks were protected by a good dense cover of grass. The channel is non-uniform in slope and cross section. The emergency spillway channel ends at a brush and tree covered slope about 75 feet downstream from the dam, (Photo 13).

There is no development in the spillway areas which would suffer damage due to flow through the spillways. High flows through the emergency spillway may overflow the protective berm onto the downstream slope of the dam.

An old pumphouse (Photo 17) was located downstream of the dam just beyond the toe of slope, (Plate 3). The pumphouse contained a pump and valves which appeared to have not been used for many years. The interior of the pumphouse was dry. Connecting piping to the pump and valves was not observable. There appeared to be no electrical connections to the pumphouse.

d. Geology. The soils surrounding the dam and reservoir consist of clayey silt to silty clay (CL-ML, to CL) to a depth of 8 inches, overlying silty clay (CL), with clayey sand (SC), clayey gravel (GC) and highly plastic clay (CH) present below a depth of 40 inches. The foundation of the dam is silty clay (CL) to clayey silt (CL-ML) as are both abutments. The emergency spillway was cut through the same material.

Based on visual examination of auger samples taken near the crest of the dam, the embankment material consists of silty clay (CL).

e. Reservoir Area. No slumping or slides of the reservoir banks were observed. The lake was noted to be clean with no siltation.

f. Downstream Channel. The principal spillway discharges to a small pond, then to the natural stream. The emergency spillway discharges to a brush and tree covered slope, then to a small pond and to the natural stream below. The natural stream is tree lined and leads to a culvert under Highways 740 and 63.

### 3.2 EVALUATION

The various deficiencies observed at the time of the inspection are not believed to represent an immediate safety hazard. They do, however, warrant monitoring and control or repair.

It is believed by the inspection team that the erosion and sloughing of the upstream face is due to wave action when the lake level is higher than the riprap. This condition will continue to worsen as long as the water level is allowed to rise above the slope protection.

The growth of trees and brush and the uncut grass, if allowed to go unchecked, could cause deterioration of the embankment. The roots of trees can loosen the embankment material and also can leave voids through which water can pass. Brush on the dam prevents inspection of the embankment and kills the smaller grasses whose roots are more effective in protecting the surface soil of the slope from erosion. The brush and tall uncut grass provides habitat for burrowing animals which can damage the embankment.

The area of seepage on the downstream slope which was observed should be monitored regularly for quality and quantity. Seepage can cause internal erosion creating cavities and underground channels, thereby weakening the embankment and/or abutments. It is believed that the wet area on the right abutment is seepage through an old spillway channel that has been filled. If so, it probably does not constitute a safety deficiency but is a nuisance.

Burrowing animals will continue to damage the embankment if no program is undertaken to eliminate them. Piping failure of the embankment has resulted in similar small earth dams due to burrowing animal damage.

## SECTION 4 - OPERATIONAL PROCEDURES

### 4.1 PROCEDURES

The pool is primarily controlled by rainfall, runoff, evaporation, transpiration, and capacity of the uncontrolled spillway.

### 4.2 MAINTENANCE OF DAM

The only maintenance evident was that the grass on the crest of the dam had been mowed and trees had been removed from under the power-line that crosses a part of the downstream slope. Mr. Platt indicated that the residents of the lake area mow the crest and that the power company has removed trees along its right of way. He also indicated that silt had been removed from the lake area by dredging.

### 4.3 MAINTENANCE OF OPERATING FACILITIES

No operating facilities exist.

### 4.4 DESCRIPTION OF ANY WARNING SYSTEM IN EFFECT

There is no existing warning system or preplanned scheme for alerting downstream residents for this dam.

### 4.5 EVALUATION

The existing maintenance effort has not been adequate in preserving the dam in a satisfactory condition.

## SECTION 5 - HYDRAULIC/HYDROLOGIC

### 5.1 EVALUATION OF FEATURES

- a. Design Data. No design data were available.
- b. Experience Data. The drainage area and lake surface area are developed from USGS Columbia (1967, Photo revised 1974), Mo. Quadrangle Map. The dam layout is from a survey made during the inspection.
- c. Visual Observations.
  - (1) The principal spillway appears to be in good condition. The lake level at the time of the inspection (El. 681.1) was below the principal spillway pipe inlet. There were no obstructions to flow in the downstream channel. The existence of the steel grate at the top of the inlet has no appreciable effect on discharge through the principal spillway.
  - (2) The emergency spillway for this dam consists of a grass-lined open channel cut through the left abutment.
  - (3) Excessive discharges through the emergency spillway could endanger the integrity of the dam, due to potential for overflow across downstream embankment slope.
- d. Overtopping Potential. The spillways will not pass the probable maximum flood without overtopping the dam. The probable maximum flood is defined as the flood discharge that may be expected from the most severe combination of critical meteorologic and hydrologic conditions that are reasonably possible in the region. The spillways will pass 10 percent of the probable maximum flood without overtopping the dam. The spillways will not pass the one percent chance flood (100-year flood) developed from a 24-hour, one percent chance rainfall, but will pass the 10 percent chance flood (10-year flood) developed from a 24-hour, 10 percent chance rainfall. According to the recommended guidelines from the Department of the Army, Office of the Chief of Engineers, a high hazard dam of small size should pass 50 to 100 percent of the probable maximum flood. Considering the downstream hazard, the appropriate spillway design flood should be 100 percent of the probable maximum flood. The portion of the estimated peak discharge of 50 percent of the probable maximum flood overtopping the dam would be 506 cfs of the total discharge from the reservoir of 592 cfs. The estimated duration of overtopping is 5.2 hours with a maximum height of 0.7 feet. The portion of the estimated peak discharge of the probable maximum flood overtopping the dam would be 1,105 cfs of the total discharge from the reservoir of 1,215 cfs. The estimated duration of overtopping is 6.4 hours with a maximum height of 1.0 feet. The embankment could be jeopardized should overtopping occur for these periods of time.



According to the St. Louis District, Corps of Engineers, the effect from rupture of the dam could extend approximately two miles downstream of the dam. Four dwellings, one building, three apartment buildings, a gasoline station, a trailer court consisting of approximately 15 trailers, and the junction of Highways 740 and 63 are located within the estimated damage zone, and lives could be lost should failure of the dam occur. Contents of the estimated downstream damage zone were verified by the inspection team. Lake Cyrene Dam and the downstream damage zone lie within the City of Columbia. Floodplain development below the dam is regulated by the city in accordance with the requirements of the Flood Insurance Program.

## SECTION 6 - STRUCTURAL STABILITY

### 6.1 EVALUATION OF STRUCTURAL STABILITY

a. Visual Observations. Visual observations of conditions which affect the structural stability of this dam are discussed in Section 3, paragraph 3.1b.

b. Design and Construction Data. No design data relating to the structural stability of the dam were found. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

c. Operating Records. No operational records exist.

d. Postconstruction Changes. According to Mr. William Platt, in 1970 a rock lined spillway channel through the right abutment was filled in with clay and a drop inlet and 18-inch pipe was installed through the dam. At a later date, the inlet crest of the drop inlet was raised by adding a section of corrugated metal pipe to the top of the drop inlet.

e. Seismic Stability. The dam is located in Seismic Zone 1 which is a zone of minor seismic risk. A properly designed and constructed earth dam using sound engineering principles and conservatism should pose no serious stability problems during earthquakes in this zone. The seismic stability of an earth dam is dependent upon a number of factors: embankment and foundation material classifications and shear strengths; abutment materials, conditions, and strengths; embankment zoning; and embankment geometry. Adequate descriptions of embankment design parameters, foundation and abutment conditions, or static stability analyses to assess the seismic stability of this embankment were not available and therefore no inferences will be made regarding the seismic stability. An assessment of the seismic stability should be included as part of the stability analysis required by the guidelines.

## SECTION 7 - ASSESSMENT/REMEDIAL MEASURES

### 7.1 DAM ASSESSMENT

a. Safety. Several conditions observed during the visual inspection by the inspection team should be corrected or monitored and controlled. These are erosion and sloughing on the upstream face, seepage on the downstream slope, the growth of and trees on both the upstream and downstream faces of the dam, and animal burrows in the embankment. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

b. Adequacy of Information. Due to the absence of engineering design data, the conclusions in this report were based only on performance history and visual conditions. The inspection team considers that these data are sufficient to support the conclusions herein. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

c. Urgency. It is the opinion of the inspection team that a program should be developed as soon as possible to implement remedial measures recommended in paragraph 7.2b. If the safety deficiencies listed in paragraph 7.1a are not corrected, they will continue to deteriorate and lead to a serious potential of failure. The item recommended in paragraph 7.2a should be pursued on a high priority basis.

d. Necessity for Phase II. The Phase I investigation does not raise any serious questions relating to the safety of the dam nor does it identify any serious dangers which would require a Phase II investigation. However, the additional analyses noted in paragraph 2.5b are necessary for compliance with the guidelines.

e. Seismic Stability. This dam is located in Seismic Zone 1. Adequate description of embankment design parameters, foundation and abutment conditions, or static stability analyses to assess the seismic stability of this embankment were not available and therefore no inferences will be made regarding the seismic stability. An assessment of the seismic stability should be included as part of the recommended stability analysis.

### 7.2 REMEDIAL MEASURES

a. Alternatives. Spillway capacity and/or the reservoir storage volume would need to be increased or the lake level would need to be permanently lowered to increase available flood storage in order to

effectively pass the spillway design flood. Spillway capacity could be increased by modifying the existing grass-lined emergency spillway or by increasing the principal spillway pipe size. The storage volume could be increased by raising the crest of the dam or by lowering the lake level by lowering the principal spillway inlet.

b. Operation and Maintenance Procedures. The following operation and maintenance procedures are recommended and should be carried out under the direction of a professional engineer experienced in the design, construction, and maintenance of earth dams.

(1) The erosion and sloughing on the upstream face should be repaired and the slope redressed. Riprap should be placed on the upstream face to an elevation above normal lake level to prevent erosion of the embankment material.

(2) The seepage areas noted during the visual inspection should be closely monitored and documented as to quantity and quality of flow. Any significant changes should be evaluated.

(3) A maintenance program should be formulated and implemented to remove and control the growth of trees on the embankments. Grass/weed cover on the embankments should be cut periodically.

(4) The animal burrows in the embankment should be repaired since they can contribute to the occurrence of piping. Control measures should be implemented to discourage animal activity in the area. The embankment slope should be monitored by a qualified engineer during repair of the embankment.

(5) Seepage and stability analyses should be performed.

(6) A detailed inspection of the dam should be made periodically and the findings of the inspections should be documented and made a matter of record. More frequent inspections may be required if additional deficiencies are observed or the severity of the reported deficiencies increase.

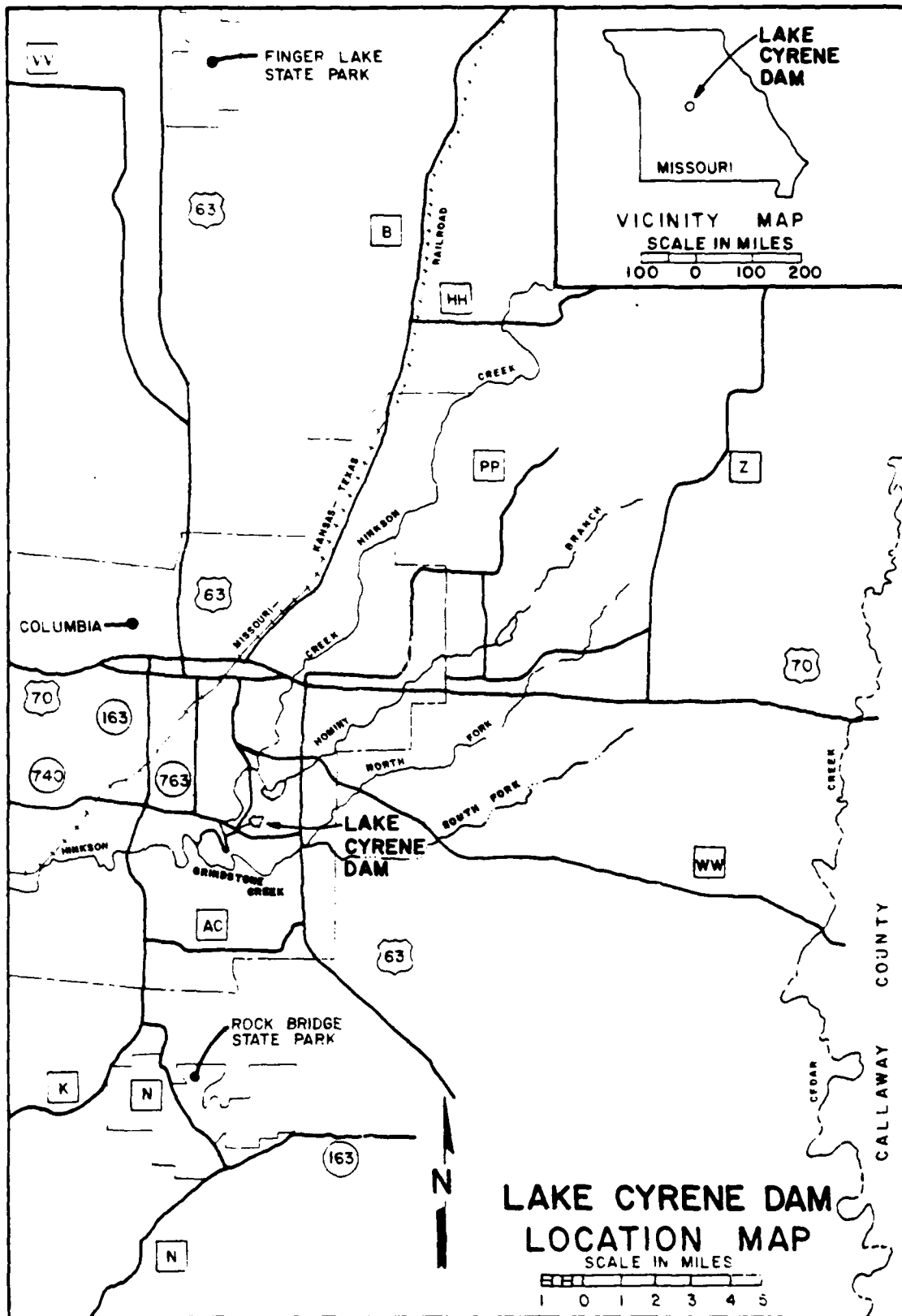


PLATE 1

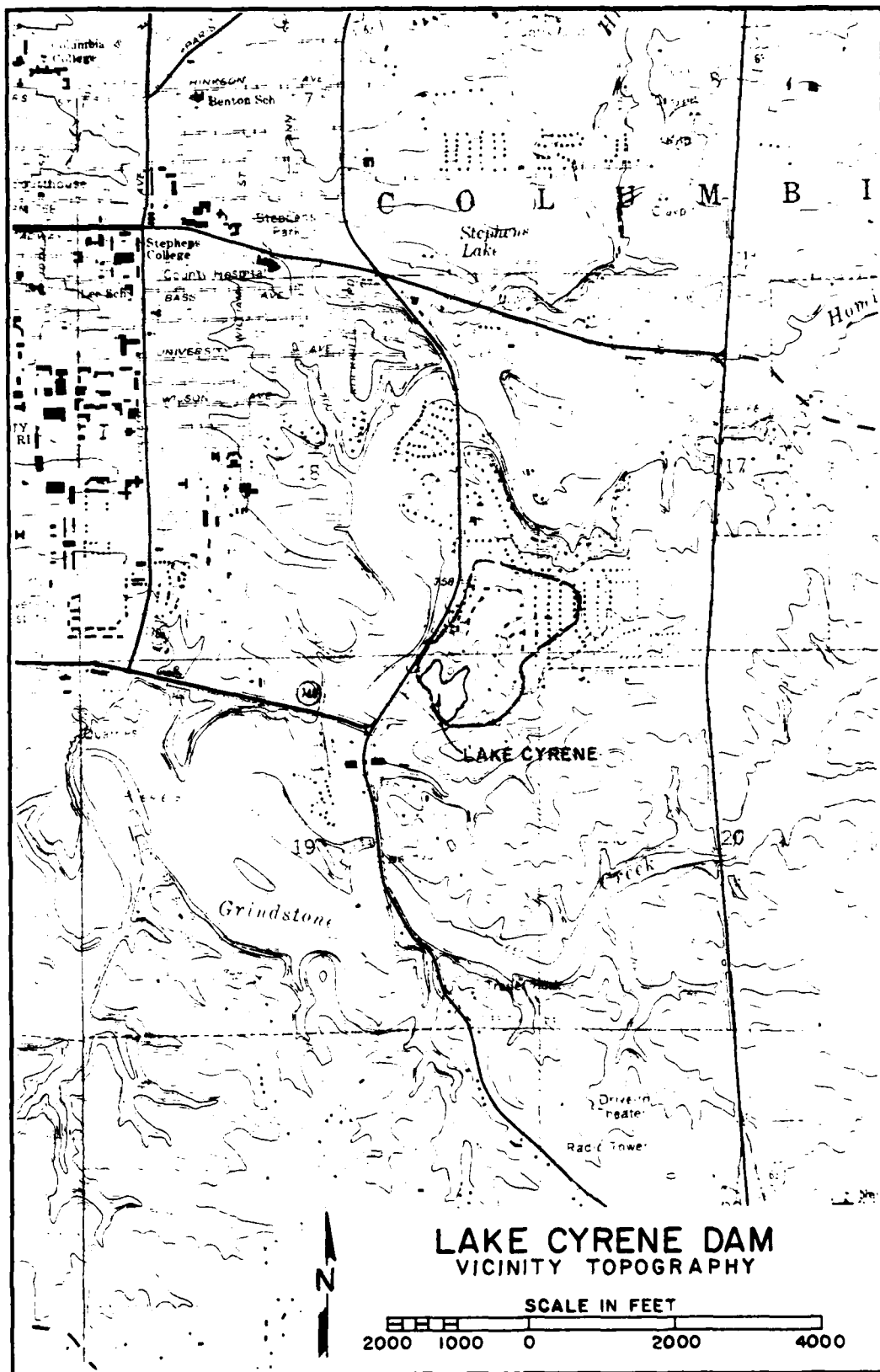
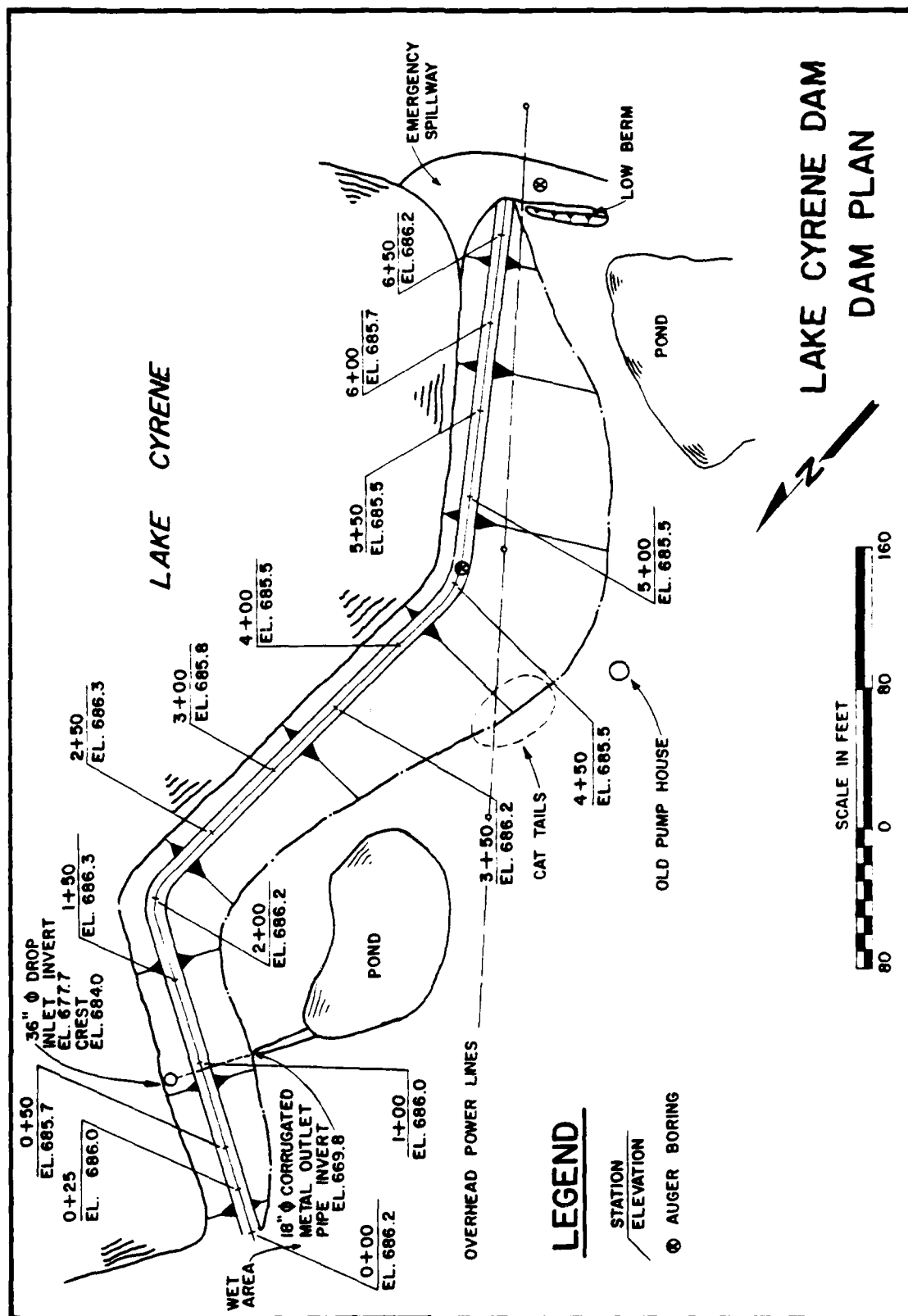
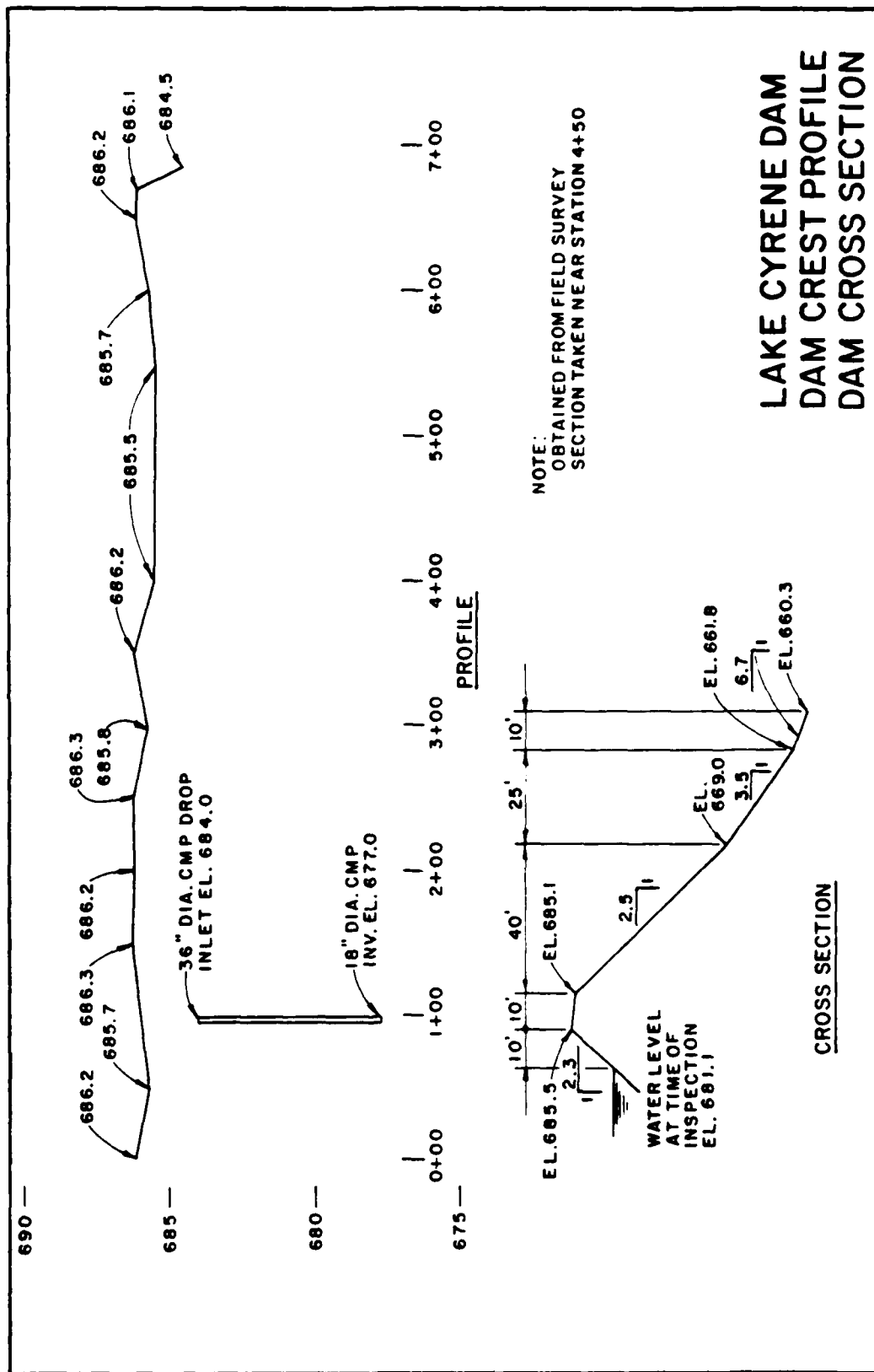
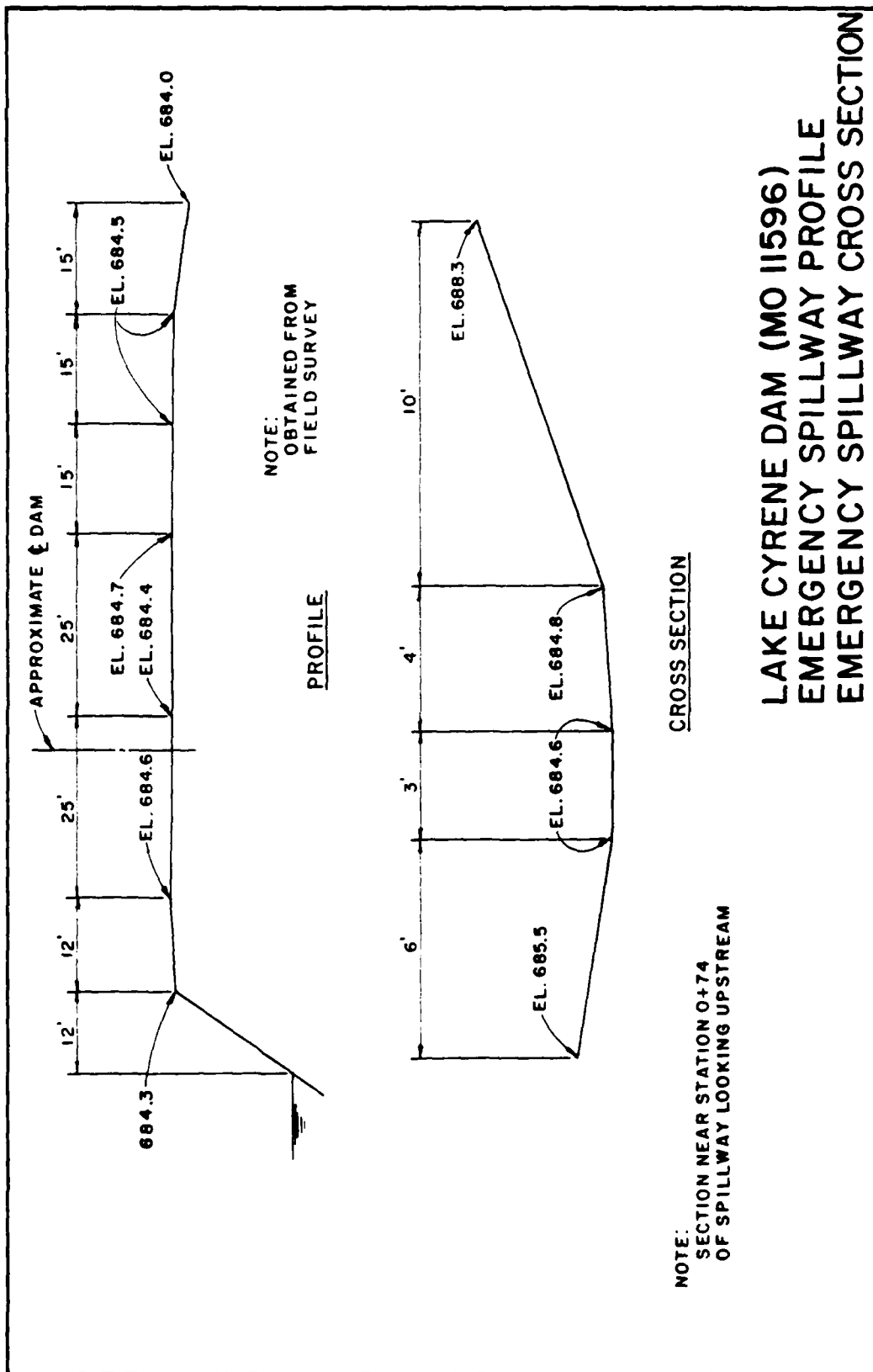


PLATE 2









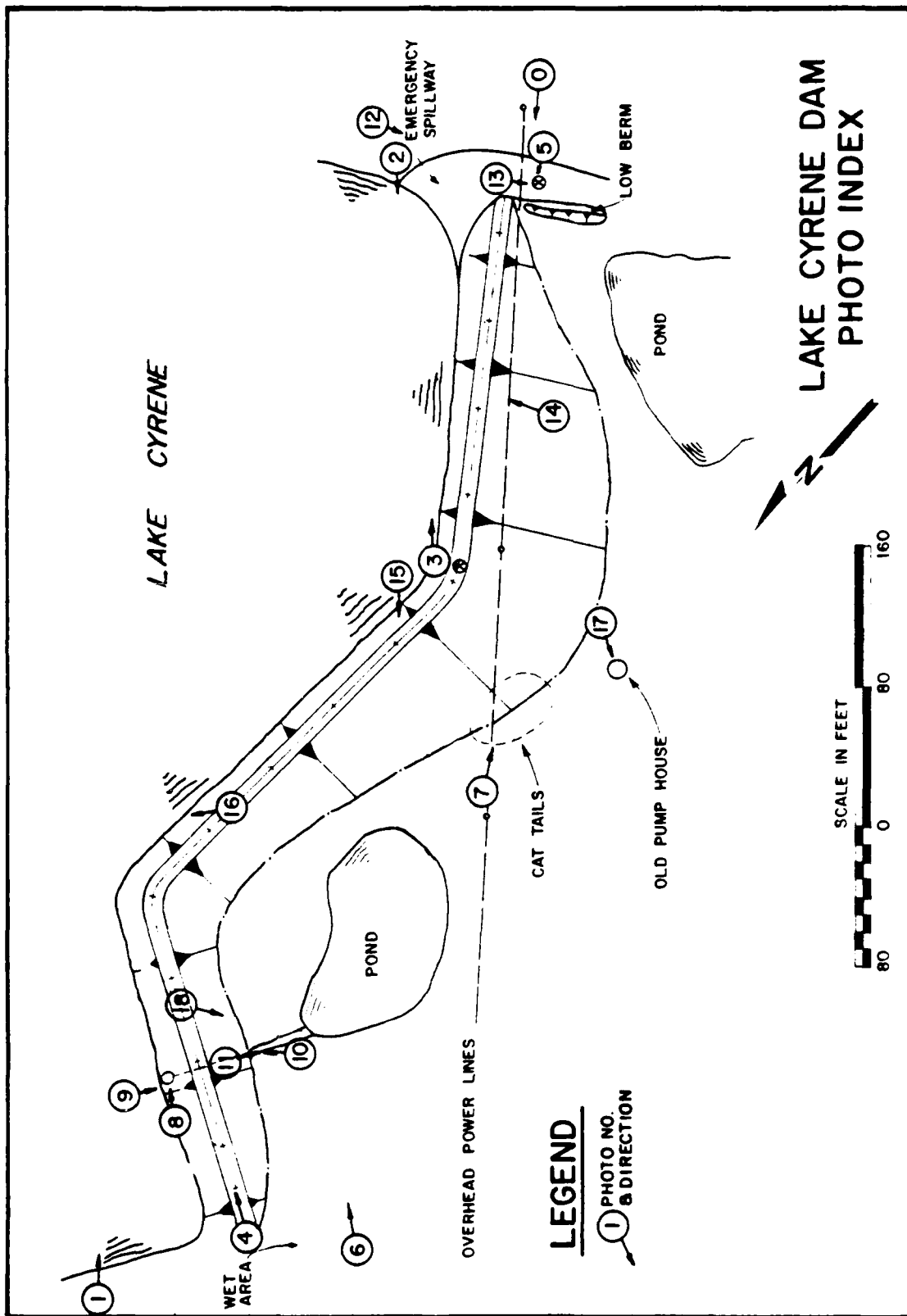


PLATE 6



PHOTO 1: UPSTREAM FACE OF DAM WEST END



PHOTO 2: UPSTREAM FACE OF DAM AT MID-DAM



PHOTO 3: UPSTREAM FACE OF DAM EAST END



PHOTO 4: CREST OF DAM LOOKING EAST



PHOTO 5: CREST OF DAM LOOKING WEST



PHOTO 6: DOWNSTREAM FACE OF DAM LOOKING EAST



PHOTO 7: DOWNSTREAM FACE OF DAM AT MIDDOWN

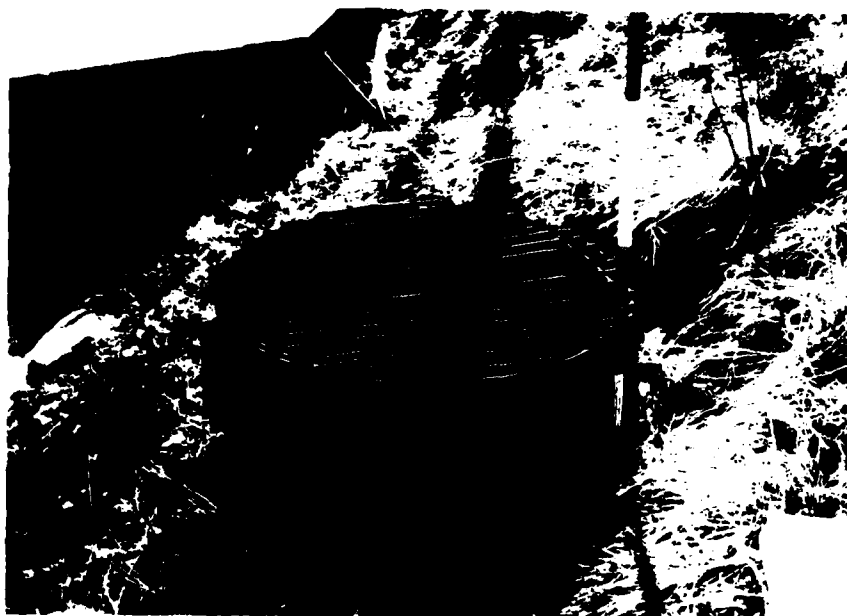


PHOTO 8: DROP INLET TO PRINCIPAL SPILLWAY

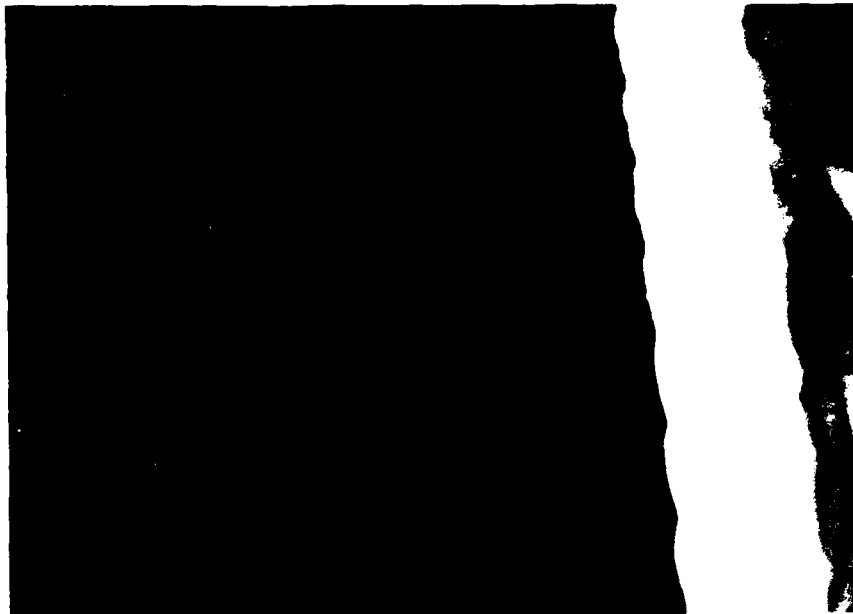


PHOTO 9: OUTLET FROM DROP INLET



PHOTO 10: DOWNSTREAM END OF PRINCIPAL SPILLWAY PIPE



PHOTO 11: PRINCIPAL SPILLWAY OUTLET AND DOWNSTREAM CHANNEL



PHOTO 12: OVERVIEW OF EMERGENCY SPILLWAY





PHOTO 13: EMERGENCY SPILLWAY CHANNEL LOOKING DOWN



PHOTO 14: ANIMAL BURROW ON DOWNSTREAM FACE OF DAM



PHOTO 15: EROSION OF UPSTREAM FACE OF DAM



PHOTO 16: UPSTREAM WATERSHED AREA



PHOTO 17: ABANDONED PUMP HOUSE DOWNSTREAM OF DAM



PHOTO 18: AREA DOWNSTREAM OF DAM

APPENDIX A  
HYDROLOGIC AND HYDRAULIC ANALYSES

## HYDROLOGIC AND HYDRAULIC ANALYSES

To determine the overtopping potential, flood routings were performed by applying the Probable Maximum Precipitation (PMP) to a synthetic unit hydrograph to develop the inflow hydrograph. The inflow hydrograph was then routed through the reservoir and spillway. The overtopping analysis was determined using the computer program HEC-1 (Dam Safety Version) (1).

The PMP was determined from regional charts prepared by the National Weather Service in "Hydrometeorological Report No. 33" (HMR-33) (2). Reduction factors were not applied. The rainfall distribution for the 24-hour PMP storm was determined according to the procedures outlined in HMR-33 and EM 1110-2-1411 (3). The Jefferson City, Missouri rainfall distribution (5 min. interval - 24 hours duration), as provided by the St. Louis District, Corps of Engineers, was used when the one percent and ten percent chance probability floods were routed through the reservoir and spillways.

The synthetic unit hydrograph for the watershed was developed by the computer program using the Soil Conservation Service (SCS) method (1,5). The parameters for the unit hydrograph are shown in Table 1. The formula from which the lag time was derived is noted in Table 1. The lag time was verified by the SCS curve number method (7).

The SCS curve number (CN) method was used in computing the infiltration losses for the rainfall-runoff relationship. The CN values used, and the result from the computer output, are shown in Table 2.

The reservoir routing was performed using the modified Puls Method. The initial reservoir pool elevation for the routing of each storm was assumed to be equivalent to the crest elevation of the principal spillway at elevation 684.0 feet m.s.l. in accordance with antecedent storm conditions AMC II, and AMC III preceding the one percent probability and probable maximum storms respectively, outlined by the U.S. Army Corps of Engineers, St. Louis District (4). The hydraulic capacity of the spillway and the storage capacity of the reservoir were defined by the elevation, surface area, storage, and discharge relationships shown in Table 3.

The rating curve for the spillway is shown in Table 4. The flow over the crest of the dam was determined using the non-level dam crest option (\$L and \$V cards) of the HEC-1 program. The program assumes critical flow over a broad-crested weir.

The result of the routing analysis indicates that a flood equivalent to a maximum of 10 percent of the PMF will not overtop the dam.

A summary of the routing analysis for different ratios of the PMF is shown in Table 5.

The computer input data and a summary of the output data are presented at the back of this appendix.

TABLE 1  
SYNTHETIC UNIT HYDROGRAPH

Parameters:

Drainage Area (A)	64 acres
Length of Watercourse (L)	0.32 miles
Difference in Elevation (H)	91 feet
Time of concentration ( $T_c$ )	0.20 hours
Lag Time ( $L_g$ )	0.13 hours
Duration (D)	1.0 min. (use 5 min.)

<u>Time (Min.) *</u>	<u>Discharge (cfs) *</u>
0	0
5	126
10	282
15	205
20	88
25	40
30	18
35	8
40	4
45	2

\* From HEC-1 computer output

FORMULAS USED:

$$T_c = (11.9 \times L^3/H)^{0.385} \quad (5)$$

$$D = 0.133 T_c$$

$$L_g = 0.6 T_c$$

TABLE 2  
RAINFALL-RUNOFF VALUES

<u>Selected Storm Event</u>	<u>Storm Duration (Hours)</u>	<u>Rainfall (Inches)</u>	<u>Runoff (Inches)</u>	<u>Loss (Inches)</u>
PMP	24	32.37	31.35	1.02
1% Probability	24	7.44	5.33	2.11
10% Probability	24	5.21	3.27	1.94

Additional Data:

- 1) The soil associations in this watershed are Union, Mandeville, and Weldon (6).  
15 percent of drainage area is hydrologic soil Group B  
60 percent of drainage area is hydrologic soil Group C  
25 percent of drainage area is hydrologic soil Group D  
100 percent of the land use was residential
- 2) SCS Runoff Curve CN = 92 (AMC III) for the PMF.
- 3) SCS Runoff Curve CN = 82 (AMC II) for the one percent and ten percent probability floods (7).

TABLE 3  
ELEVATION, SURFACE AREA, STORAGE, AND DISCHARGE RELATIONSHIPS

<u>Elevation (feet-MSL)</u>	<u>Lake Surface Area (acres)</u>	<u>Lake Storage (acre-ft)</u>	<u>Spillway Discharge (cfs)</u>
*684.0	6.3	50	0
**684.6	6.6	53	11
***685.5	6.8	60	50
685.9	6.9	63	70
686.2	7.0	65	86
686.5	7.2	67	110

\*Principal Spillway Inlet Crest Elevation  
\*\*Emergency Spillway Crest Elevation  
\*\*\*Top of Dam Elevation

The relationships in Table 3 were developed from the Columbia, Missouri 7.5 minute quadrangle map and field measurements.

TABLE 4  
SPILLWAY RATING CURVE

<u>Reservoir Elevation (ft)</u>	<u>Principal Spillway Discharge (cfs)</u>	<u>Emergency Spillway Discharge (cfs)</u>	<u>Total Spillway Discharge (cfs)</u>
*684.0	0	-	0
**684.6	11	0	11
685.0	21	5	26
***685.5	22	28	50
685.9	22	43	65
686.2	22	64	86
686.5	23	87	110

\*Principal Spillway Inlet Crest Elevation

\*\*Emergency Spillway Crest Elevation

\*\*\*Top of Dam Elevation

METHOD USED:

Principal spillway release rates are based on the orifice equations of flow through a pipe culvert (8)

Emergency spillway release rates are based on the weir flow equation:

$$Q = CL \times (H^{3/2})$$

C = Coefficient of Discharge = 3.2 (9)

L = Length of Weir Crest = 9.4 (feet)

H = Head over Crest (feet)



TABLE 5  
RESULTS OF FLOOD ROUTINGS

Ratio of PMF	Peak Inflow (cfs)	Peak Lake Elevation (ft.-msl)	Total Storage (ac.-ft.)	Peak Outflow (cfs)	Depth (ft.) Over Top of Dam	Duration of Over- topping (hrs)
-	0	*684.0	50	0	-	-
0.10	131	685.1	57	30	-	-
0.15	197	685.6	61	57	0.05	0.05
0.50	656	686.2	65	592	0.71	5.2
1.00	1,312	686.5	67	1,215	1.01	6.4

\* Principal spillway inlet crest elevation

- (1) U.S. Army Corps of Engineers, Hydrologic Engineering Center, Flood Hydrograph Package (HEC-1), Dam Safety Version, July 1978, Modification April 1980, Davis, California.
- (2) HMR 33, Seasonal Variations of Probable Maximum Precipitation, East of the 105th Meridian for Areas 10 to 1000 Square Miles and Durations from 6 to 48 Hours, U.S. Department of Commerce, NOAA, National Weather Service, 1956.
- (3) EM-1110-2-1411, Standard Project Flood Determinations, U.S. Army Corps of Engineers, 26 March 1952.
- (4) U.S. Army Corps of Engineers, St. Louis District, Hydrologic/Hydraulic Standards, Phase I Safety Inspection of Non-Federal Dams, 22 August 1980.
- (5) U.S. Department of the Interior, Bureau of Reclamation, Design of Small Dams, 1974, Washington, D.C.
- (6) U.S. Department of Agriculture, Soil Conservation Service, Soil Survey of Boone County Missouri, 1962.
- (7) U.S. Department of Agriculture, Soil Conservation Service, National Engineering Handbook, Section 4, Hydrology, August 1972.
- (8) Ernest F. Brater and Horace Williams King, Handbook of Hydraulics, New York: McGraw-Hill Book Co., Inc., 6th Edition, 1976, pp. 4-22-4-24.
- (9) U.S. Department of the Interior, Geological Survey, Techniques of Water Resources Investigations of the United States Geological Survey, Chapter A5, 1967.



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DATA SAFETY VERSION JULY 1978  
LAST MODIFICATION 21 APR 56

DISCUSS DAM INSPECTIONS  
LAKE CLEVELAND  
FMS AND RATIOS

[illegible]

MULTI-PLAN ANALYSES TO BE PERFORMED

RPLCS=	.10	.15	NPLAH= 9 NPTIO= 9 LRTIO= 9			.46	.38	1.06
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SUG-AREA RUNOFF COMPUTATION

COMPUTE INFLC=HYDROGRAPH

ISTAU	LCOPP	IFCON	ISTAPL	JPLY	JPRY	INAME	ISTAGE	IAUTO
HEAD	0	0	0	0	0	-	1	0

INVD6	INUM	YAFEA	SNAP	TRSEP	TRSPC	RATIO	ISNOB	ISAME	LOCAL
1	2	-12	CC	1P	1-30	.000	0	1	0

REF ID: A7A

SPARE	PAS	86	87	824	848	872	896
24.54	101.00	120.00	130.00	150	170	190	210

**LOSS DATA**

ALSOFT	STGRN	OLIVE	RYTOL	GRAIN	STKRS	RTIOK	STAYL	CNSTL	ALSPH	RTIMP
0	.00	.00	1.00	.00	.00	1.00	-1.00	-92.00	.00	.00

CURVE NO = -02.29 WEIENSS = -1.01 PULS PER SEC = 02.00

UNIT HYDROGRAPH DATA

REF ID: A6015233

001-20100 071-00100 001-20100

TIME INCREMENT TOO LARGE--(406 15 57 146/22)

UNIT HYDROGRAPH 1C END OF PERIOD ORIGINATES, TC: .63 HOURS, LAG = .13 VOL = 1.04

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DATE 23 MAR 81
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CLOD HYDROGRAM PACKAGE - MEC-1
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PROGRAM M21/02-1V
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TIME 14:19:17
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CASE
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149-07-748104 1102

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1.01	5.30	66	.01	.01	.01	7.	1.01	17.30	210	.29	.29	.00	179.
1.01	5.35	67	.01	.01	.01	6.	1.01	17.35	211	.29	.29	.00	179.
1.01	5.40	68	.01	.01	.01	6.	1.01	17.40	212	.29	.29	.00	178.
1.01	5.45	69	.01	.01	.01	6.	1.01	17.45	213	.29	.29	.00	178.
1.01	5.50	70	.01	.01	.01	6.	1.01	17.50	214	.29	.29	.00	178.
1.01	5.55	71	.01	.01	.01	6.	1.01	17.55	215	.29	.29	.00	178.
1.01	5.60	72	.01	.01	.01	6.	1.01	17.60	216	.29	.29	.00	178.
1.01	5.65	73	.01	.01	.01	12.	1.01	17.65	217	.02	.02	.00	152.
1.01	5.70	74	.01	.01	.01	12.	1.01	17.70	218	.02	.02	.00	93.
1.01	5.75	75	.01	.01	.01	24.	1.01	17.75	219	.02	.02	.00	59.
1.01	5.80	76	.01	.01	.01	30.	1.01	17.80	220	.02	.02	.00	31.
1.01	5.85	77	.01	.01	.01	35.	1.01	17.85	221	.02	.02	.00	23.
1.01	5.90	78	.01	.01	.01	40.	1.01	17.90	222	.02	.02	.00	19.
1.01	5.95	79	.01	.01	.01	41.	1.01	17.95	223	.02	.02	.00	17.
1.01	6.00	80	.01	.01	.01	42.	1.01	18.00	224	.02	.02	.00	17.
1.01	6.05	81	.01	.01	.01	42.	1.01	18.05	225	.02	.02	.00	16.
1.01	6.10	82	.01	.01	.01	43.	1.01	18.10	226	.02	.02	.00	16.
1.01	6.15	83	.01	.01	.01	43.	1.01	18.15	227	.02	.02	.00	16.
1.01	6.20	84	.01	.01	.01	44.	1.01	18.20	228	.02	.02	.00	16.
1.01	6.25	85	.01	.01	.01	44.	1.01	18.25	229	.02	.02	.00	16.
1.01	6.30	86	.01	.01	.01	45.	1.01	18.30	230	.02	.02	.00	16.
1.01	6.35	87	.01	.01	.01	45.	1.01	18.35	231	.02	.02	.00	16.
1.01	6.40	88	.01	.01	.01	45.	1.01	18.40	232	.02	.02	.00	16.
1.01	6.45	89	.01	.01	.01	45.	1.01	18.45	233	.02	.02	.00	16.
1.01	6.50	90	.01	.01	.01	46.	1.01	18.50	234	.02	.02	.00	16.
1.01	6.55	91	.01	.01	.01	46.	1.01	18.55	235	.02	.02	.00	16.
1.01	6.60	92	.01	.01	.01	46.	1.01	18.60	236	.02	.02	.00	16.
1.01	6.65	93	.01	.01	.01	47.	1.01	18.65	237	.02	.02	.00	16.
1.01	6.70	94	.01	.01	.01	47.	1.01	18.70	238	.02	.02	.00	16.
1.01	6.75	95	.01	.01	.01	47.	1.01	18.75	239	.02	.02	.00	16.
1.01	6.80	96	.01	.01	.01	47.	1.01	18.80	240	.02	.02	.00	16.
1.01	6.85	97	.01	.01	.01	47.	1.01	18.85	241	.02	.02	.00	16.
1.01	6.90	98	.01	.01	.01	47.	1.01	18.90	242	.02	.02	.00	16.
1.01	6.95	99	.01	.01	.01	47.	1.01	18.95	243	.02	.02	.00	16.
1.01	7.00	100	.01	.01	.01	48.	1.01	19.00	244	.02	.02	.00	16.
1.01	7.05	101	.01	.01	.01	48.	1.01	19.05	245	.02	.02	.00	16.
1.01	7.10	102	.01	.01	.01	48.	1.01	19.10	246	.02	.02	.00	16.
1.01	7.15	103	.01	.01	.01	48.	1.01	19.15	247	.02	.02	.00	16.
1.01	7.20	104	.01	.01	.01	48.	1.01	19.20	248	.02	.02	.00	16.
1.01	7.25	105	.01	.01	.01	48.	1.01	19.25	249	.02	.02	.00	16.
1.01	7.30	106	.01	.01	.01	48.	1.01	19.30	250	.02	.02	.00	16.
1.01	7.35	107	.01	.01	.01	48.	1.01	19.35	251	.02	.02	.00	16.
1.01	7.40	108	.01	.01	.01	48.	1.01	19.40	252	.02	.02	.00	16.
1.01	7.45	109	.01	.01	.01	48.	1.01	19.45	253	.02	.02	.00	16.
1.01	7.50	110	.01	.01	.01	48.	1.01	19.50	254	.02	.02	.00	16.
1.01	7.55	111	.01	.01	.01	48.	1.01	19.55	255	.02	.02	.00	16.
1.01	7.60	112	.01	.01	.01	48.	1.01	19.60	256	.02	.02	.00	16.
1.01	7.65	113	.01	.01	.01	48.	1.01	19.65	257	.02	.02	.00	16.
1.01	7.70	114	.01	.01	.01	48.	1.01	19.70	258	.02	.02	.00	16.
1.01	7.75	115	.01	.01	.01	48.	1.01	19.75	259	.02	.02	.00	16.
1.01	7.80	116	.01	.01	.01	48.	1.01	19.80	260	.02	.02	.00	16.
1.01	7.85	117	.01	.01	.01	48.	1.01	19.85	261	.02	.02	.00	16.
1.01	7.90	118	.01	.01	.01	48.	1.01	19.90	262	.02	.02	.00	16.
1.01	7.95	119	.01	.01	.01	48.	1.01	19.95	263	.02	.02	.00	16.
1.01	8.00	120	.01	.01	.01	48.	1.01	20.00	264	.02	.02	.00	16.
1.01	8.05	121	.01	.01	.01	48.	1.01	20.05	265	.02	.02	.00	16.
1.01	8.10	122	.01	.01	.01	48.	1.01	20.10	266	.02	.02	.00	16.
1.01	8.15	123	.01	.01	.01	48.	1.01	20.15	267	.02	.02	.00	16.
1.01	8.20	124	.01	.01	.01	48.	1.01	20.20	268	.02	.02	.00	16.
1.01	8.25	125	.01	.01	.01	48.	1.01	20.25	269	.02	.02	.00	16.
1.01	8.30	126	.01	.01	.01	48.	1.01	20.30	270	.02	.02	.00	16.
1.01	8.35	127	.01	.01	.01	48.	1.01	20.35	271	.02	.02	.00	16.
1.01	8.40	128	.01	.01	.01	48.	1.01	20.40	272	.02	.02	.00	16.
1.01	8.45	129	.01	.01	.01	48.	1.01	20.45	273	.02	.02	.00	16.
1.01	8.50	130	.01	.01	.01	48.	1.01	20.50	274	.02	.02	.00	16.
1.01	8.55	131	.01	.01	.01	48.	1.01	20.55	275	.02	.02	.00	16.
1.01	8.60	132	.01	.01	.01	48.	1.01	20.60	276	.02	.02	.00	16.
1.01	8.65	133	.01	.01	.01	48.	1.01	20.65	277	.02	.02	.00	16.
1.01	8.70	134	.01	.01	.01	48.	1.01	20.70	278	.02	.02	.00	16.
1.01	8.75	135	.01	.01	.01	48.	1.01	20.75	279	.02	.02	.00	16.
1.01	8.80	136	.01	.01	.01	48.	1.01	20.80	280	.02	.02	.00	16.
1.01	8.85	137	.01	.01	.01	48.	1.01	20.85	281	.02	.02	.00	16.
1.01	8.90	138	.01	.01	.01	48.	1.01	20.90	282	.02	.02	.00	16.
1.01	8.95	139	.01	.01	.01	48.	1.01	20.95	283	.02	.02	.00	16.
1.01	9.00	140	.01	.01	.01	48.	1.01	21.00	284	.02	.02	.00	16.
1.01	9.05	141	.01	.01	.01	48.	1.01	21.05	285	.02	.02	.00	16.

1.00	8.50	106	.07	.06	.00	42.	1.01	20.50	250	.02	.02	.00	16.
1.01	9.00	107	.07	.06	.00	48.	1.01	20.55	251	.02	.02	.00	16.
1.02	9.50	108	.07	.06	.00	48.	1.01	21.00	252	.02	.02	.00	16.
1.03	9.55	109	.07	.06	.00	48.	1.01	21.05	253	.02	.02	.00	16.
1.04	9.60	110	.07	.06	.00	49.	1.01	21.10	254	.02	.02	.00	16.
1.05	9.65	111	.07	.06	.00	49.	1.01	21.15	255	.02	.02	.00	16.
1.06	9.70	112	.07	.06	.00	49.	1.01	21.20	256	.02	.02	.00	16.
1.07	9.75	113	.07	.06	.00	49.	1.01	21.25	257	.02	.02	.00	16.
1.08	9.80	114	.07	.06	.00	49.	1.01	21.30	258	.02	.02	.00	16.
1.09	9.85	115	.07	.06	.00	49.	1.01	21.35	259	.02	.02	.00	16.
1.10	9.90	116	.07	.06	.00	49.	1.01	21.40	260	.02	.02	.00	16.
1.11	9.95	117	.07	.06	.00	49.	1.01	21.45	261	.02	.02	.00	16.
1.12	10.00	118	.07	.06	.00	49.	1.01	21.50	262	.02	.02	.00	16.
1.13	10.05	119	.07	.06	.00	49.	1.01	21.55	263	.02	.02	.00	16.
1.14	10.10	120	.07	.06	.00	49.	1.01	22.00	264	.02	.02	.00	16.
1.15	10.15	121	.07	.06	.00	49.	1.01	22.05	265	.02	.02	.00	16.
1.16	10.20	122	.07	.06	.00	49.	1.01	22.10	266	.02	.02	.00	16.
1.17	10.25	123	.07	.06	.00	49.	1.01	22.15	267	.02	.02	.00	16.
1.18	10.30	124	.07	.06	.00	49.	1.01	22.20	268	.02	.02	.00	16.
1.19	10.35	125	.07	.06	.00	49.	1.01	22.25	269	.02	.02	.00	16.
1.20	10.40	126	.07	.06	.00	49.	1.01	22.30	270	.02	.02	.00	16.
1.21	10.45	127	.07	.06	.00	49.	1.01	22.35	271	.02	.02	.00	16.
1.22	10.50	128	.07	.06	.00	49.	1.01	22.40	272	.02	.02	.00	16.
1.23	10.55	129	.07	.06	.00	49.	1.01	22.45	273	.02	.02	.00	16.
1.24	10.60	130	.07	.06	.00	50.	1.01	22.50	274	.02	.02	.00	16.
1.25	10.65	131	.07	.06	.00	50.	1.01	22.55	275	.02	.02	.00	16.
1.26	10.70	132	.07	.06	.00	50.	1.01	23.00	276	.02	.02	.00	16.
1.27	10.75	133	.07	.06	.00	50.	1.01	23.05	277	.02	.02	.00	16.
1.28	10.80	134	.07	.06	.00	50.	1.01	23.10	278	.02	.02	.00	16.
1.29	10.85	135	.07	.06	.00	50.	1.01	23.15	279	.02	.02	.00	16.
1.30	10.90	136	.07	.06	.00	50.	1.01	23.20	280	.02	.02	.00	16.
1.31	10.95	137	.07	.06	.00	50.	1.01	23.25	281	.02	.02	.00	16.
1.32	11.00	138	.07	.06	.00	50.	1.01	23.30	282	.02	.02	.00	16.
1.33	11.05	139	.07	.06	.00	50.	1.01	23.35	283	.02	.02	.00	16.
1.34	11.10	140	.07	.06	.00	50.	1.01	23.40	284	.02	.02	.00	16.
1.35	11.15	141	.07	.06	.00	50.	1.01	23.45	285	.02	.02	.00	16.
1.36	11.20	142	.07	.06	.00	50.	1.01	23.50	286	.02	.02	.00	16.
1.37	11.25	143	.07	.06	.00	50.	1.01	23.55	287	.02	.02	.00	16.
1.38	11.30	144	.07	.06	.00	50.	1.02	23.60	288	.02	.02	.00	16.

SUP 32.37 31.75 1.02 24233.  
 ( 822.3 ) ( 796.3 ) ( 26.3 ) ( 606.20 )

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1312.	267.	84.	84.	24229.
17.	8.	2.	2.	864.
	24.38	31.30	31.30	31.10
	679.83	795.13	795.13	795.13
	173.	167.	167.	167.
	164.	276.	276.	276.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 1

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
171.	27.	8.	8.	2423.
4.	1.	0.	0.	60.
	2.49	3.17	3.17	3.13
	63.18	79.51	79.51	79.51

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 2				
AC-FT	12.	17.	17.	
THOUS CU M	16.	21.	21.	21.
PEAK	197.			
6-HOUR	4.2	13.	13.	343.
24-HOUR	1.	6.	0.	10.
72-HOUR	3.77	4.20	4.20	4.20
TOTAL VOLUME	94.77	119.27	119.27	119.27
CFS				
CMS				
INCHES				
AC-FT	26.	25.	25.	25.
THOUS CU M	25.	25.	25.	25.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 3				
AC-FT	12.	17.	17.	
THOUS CU M	16.	21.	21.	21.
PEAK	262.			
6-HOUR	53.	17.	17.	494.
24-HOUR	2.	0.	0.	117.
72-HOUR	6.98	6.26	6.26	6.26
TOTAL VOLUME	126.37	159.03	159.03	159.03
CFS				
CMS				
INCHES				
AC-FT	27.	23.	23.	23.
THOUS CU M	23.	23.	23.	23.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 4				
AC-FT	12.	17.	17.	
THOUS CU M	16.	21.	21.	21.
PEAK	328.			
6-HOUR	47.	21.	21.	657.
24-HOUR	2.	1.	1.	172.
72-HOUR	6.22	7.83	7.83	7.83
TOTAL VOLUME	157.66	198.79	198.79	198.79
CFS				
CMS				
INCHES				
AC-FT	43.	42.	42.	42.
THOUS CU M	41.	51.	51.	51.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 5				
AC-FT	12.	17.	17.	
THOUS CU M	16.	21.	21.	21.
PEAK	39.			
6-HOUR	20.	25.	25.	726.
24-HOUR	2.	1.	1.	266.
72-HOUR	7.46	9.30	9.30	9.30
TOTAL VOLUME	189.55	228.54	228.54	228.54
CFS				
CMS				
INCHES				
AC-FT	42.	50.	50.	50.
THOUS CU M	49.	62.	62.	62.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 6				
AC-FT	12.	17.	17.	
THOUS CU M	16.	21.	21.	21.
PEAK	459.			
6-HOUR	84.	29.	29.	848.
24-HOUR	2.	1.	1.	266.
72-HOUR	7.46	9.30	9.30	9.30
TOTAL VOLUME	189.55	228.54	228.54	228.54
CFS				
CMS				
INCHES				
AC-FT	42.	50.	50.	50.
THOUS CU M	49.	62.	62.	62.



# HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 6

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
657.	54.	79.	79.	8482.
13.	2.	1.	1.	240.
	8.71	10.96	10.96	10.96
	221.14	278.29	278.29	278.29
	46.	52.	52.	52.
	57.	72.	72.	72.

BLACK & VEATCH  
FLOOD HYDROGRAPH PACKAGE - HEC-1  
PROJECT 94571 DATE 23 MAR 81 PAGE 8  
PROGRAM 421/C2-1V TIME 14:19:17 CASE

CFS  
CMS  
INCHES  
MM  
AC-FT  
THOUS CU M

# HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 7

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
525.	107.	74.	14.	9492.
15.	3.	1.	1.	274.
	9.9	12.52	12.52	12.52
	272.71	318.05	318.05	318.05
	43.	67.	67.	67.
	65.	82.	82.	82.

# HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 8

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
675.	174.	42.	42.	12115.
19.	4.	1.	1.	367.
	12.44	15.05	15.05	15.05
	315.91	397.56	397.56	397.56
	60.	73.	73.	73.
	82.	103.	103.	103.

# HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 9

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1312.	287.	84.	84.	24229.
17.	8.	2.	2.	484.
	24.59	31.30	31.30	31.30
	671.87	795.13	795.13	795.13
	123.	147.	147.	147.
	164.	206.	206.	206.

CFS  
CMS  
INCHES  
MM  
AC-FT  
THOUS CU M

## HYDROGRAPH ROUTING

### ROUTE HYDROGRAPH THROUGH DAM

STATION	ICOMP	IFROM	ITYPE	JPLT	JPRP	INAME	ISTAGE	IAUTO
DAM	1	1	0	0	0	0	0	0
			ROUTING DATA					

















CFS 12.15 4.75 92.0 23659.  
 CFS 8.0 2.0 4.0 167.  
 INCHES 20.36 20.36 20.36  
 INCHES 771.16 771.16 771.16  
 AC-FT 178 162 162  
 INCHES LUM 142 200 200

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
 AREA IN SQUARE FEET (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN RATIO	1	RATIO 2	3	RATIO 4	5	RATIO 6	7	RATIO 8	9
				.10	.15	.20	.25	.30	.35	.40	.45	1.00
HYDROGRAPH AT	HEAD	.10	1	131.	197.	262.	318.	394.	450.	525.	656.	1312.
	(	.20	(	3.72)(	5.57)(	7.47)(	9.29)(	11.15)(	13.00)(	14.86)(	18.52)(	37.16)
ROUTER TO	DAM	.10	1	35.	57.	147.	239.	325.	395.	462.	592.	1215.
	(	.20	(	.86)(	1.41)(	4.17)(	6.74)(	9.21)(	11.14)(	13.06)(	16.78)(	34.41)

SUPPLY OF DAM SAFETY ANALYSIS

PLAN 1 .....									
RATIO CF PMF	MAXIMUM RESERVOIR WATER ELEVATION	ELEVATION STORAGE OUTFLOW		INITIAL VALUE		SPILLWAY CREST		TOP OF DAM	
		OUTFLOW		50. 0.		684.00 55. 0.		685.50 73. 49.	
		MAXIMUM DEPTH AFTER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS		
.10	675.12	.60	57.	30.	.00	16.17	.00		
.15	685.55	.65	61.	57.	.50	16.08	.00		
.20	695.79	.68	64.	147.	1.58	15.88	.00		
.25	695.91	.41	63.	235.	2.42	15.75	.00		
.30	696.11	.51	64.	325.	3.33	15.77	.00		
.35	696.07	.57	64.	351.	3.62	15.75	.00		
.40	696.12	.62	64.	402.	4.50	15.75	.00		
.45	696.21	.71	65.	592.	5.17	15.75	.00		
.50	696.51	1.01	67.	1215.	6.42	15.75	.00		

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1  A  B  C  D  E  F  G  H  I  J  K  L  M  N  O  P  Q  R  S  T  U  V  W  X  Y  Z
2  1  2  3  4  5  6  7  8  9  10 11 12 13 14 15 16 17 18 19 20 21 22 23 24
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12 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220
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31	186.0				
32	186.5				
33	187.0	215.	374.	525.	670.
34	187.5	215.	380.	666.2	850.
35	188.0	215.			
36	188.5	215.			
37	189.0	215.			
38	189.5	215.			
39	190.0	215.			
40	190.5	215.			

CASB, A SYSL-000D. 7422-114-30  
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**CASE, T 11-**

**CASG, P 13.**

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Year	Number of cases	Number of deaths
1990	10	1
1991	12	2
1992	15	3
1993	18	4
1994	20	5
1995	22	6
1996	25	7
1997	28	8
1998	30	9
1999	32	10
2000	35	11
2001	38	12
2002	40	13
2003	42	14
2004	45	15
2005	48	16
2006	50	17
2007	52	18
2008	55	19
2009	58	20
2010	60	21
2011	62	22
2012	65	23
2013	68	24
2014	70	25
2015	72	26
2016	75	27
2017	78	28
2018	80	29
2019	82	30
2020	85	31
2021	88	32
2022	90	33
2023	92	34
2024	95	35
2025	98	36
2026	100	37
2027	102	38
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2032	115	43
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2051	162	62
2052	165	63
2053	168	64
2054	170	65
2055	172	66
2056	175	67
2057	178	68
2058	180	69
2059	182	70
2060	185	71
2061	188	72
2062	190	73
2063	192	74
2064	195	75
2065	198	76
2066	200	77
2067	202	78
2068	205	79
2069	208	80
2070	210	81
2071	212	82
2072	215	83
2073	218	84
2074	220	85
2075	222	86
2076	225	87
2077	228	88
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2080	235	91
2081	238	92
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2092	265	103
2093	268	104
2094	270	105
2095	272	106
2096	275	107
2097	278	108
2098	280	109
2099	282	110
2100	285	111
2101	288	112
2102	290	113
2103	292	114
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2106	300	117
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PROJECT 0657:      DATE 25 MAR 81  PAGE 1
PROGRAM M21/02-1A  TIME 13:17:46  CASE

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1.01	2.10	26	.01	.00	.00	1.01	14.10	170	.00	.02	.00	19.
1.01	2.15	27	.01	.00	.00	1.01	14.15	171	.00	.02	.00	19.
1.01	2.20	28	.01	.00	.00	1.01	14.20	172	.00	.02	.00	19.
1.01	2.25	29	.01	.00	.00	1.01	14.25	173	.00	.02	.00	19.
1.01	2.30	30	.01	.00	.00	1.01	14.30	174	.00	.02	.00	19.
1.01	2.35	31	.01	.00	.00	1.01	14.35	175	.00	.02	.00	19.
1.01	2.40	32	.01	.00	.00	1.01	14.40	176	.00	.02	.00	19.
1.01	2.45	33	.01	.00	.00	1.01	14.45	177	.00	.02	.00	19.
1.01	2.50	34	.01	.00	.00	1.01	14.50	178	.00	.02	.00	19.
1.01	2.55	35	.01	.00	.00	1.01	14.55	179	.00	.02	.00	19.
1.01	2.60	36	.01	.00	.00	1.01	15.00	180	.00	.02	.00	19.
1.01	2.65	37	.01	.00	.00	1.01	15.05	181	.00	.02	.00	19.
1.01	2.70	38	.01	.00	.00	1.01	15.10	182	.00	.02	.00	19.
1.01	2.75	39	.01	.00	.00	1.01	15.15	183	.00	.02	.00	19.
1.01	2.80	40	.01	.00	.00	1.01	15.20	184	.00	.02	.00	19.
1.01	2.85	41	.01	.00	.00	1.01	15.25	185	.00	.02	.00	19.
1.01	2.90	42	.01	.00	.00	1.01	15.30	186	.00	.02	.00	19.
1.01	2.95	43	.01	.00	.00	1.01	15.35	187	.00	.02	.00	19.
1.01	3.00	44	.01	.00	.00	1.01	15.40	188	.00	.02	.00	19.
1.01	3.05	45	.01	.00	.00	1.01	15.45	189	.00	.02	.00	19.
1.01	3.10	46	.01	.00	.00	1.01	15.50	190	.00	.02	.00	19.
1.01	3.15	47	.01	.00	.00	1.01	15.55	191	.00	.02	.00	19.
1.01	3.20	48	.01	.00	.00	1.01	16.00	192	.00	.02	.00	19.
1.01	3.25	49	.01	.00	.00	1.01	16.05	193	.00	.02	.00	19.
1.01	3.30	50	.01	.00	.00	1.01	16.10	194	.00	.02	.00	19.
1.01	3.35	51	.01	.00	.00	1.01	16.15	195	.00	.02	.00	19.
1.01	3.40	52	.01	.00	.00	1.01	16.20	196	.00	.02	.00	19.
1.01	3.45	53	.01	.00	.00	1.01	16.25	197	.00	.02	.00	19.
1.01	3.50	54	.01	.00	.00	1.01	16.30	198	.00	.02	.00	19.
1.01	3.55	55	.01	.00	.00	1.01	16.35	199	.00	.02	.00	19.
1.01	3.60	56	.01	.00	.00	1.01	16.40	200	.00	.02	.00	19.
1.01	3.65	57	.01	.00	.00	1.01	16.45	201	.00	.02	.00	19.
1.01	3.70	58	.01	.00	.00	1.01	16.50	202	.00	.02	.00	19.
1.01	3.75	59	.01	.00	.00	1.01	16.55	203	.00	.02	.00	19.
1.01	3.80	60	.01	.00	.00	1.01	17.00	204	.00	.02	.00	19.
1.01	3.85	61	.01	.00	.00	1.01	17.05	205	.00	.02	.00	19.
1.01	3.90	62	.01	.00	.00	1.01	17.10	206	.00	.02	.00	19.
1.01	3.95	63	.01	.00	.00	1.01	17.15	207	.00	.02	.00	19.
1.01	4.00	64	.01	.00	.00	1.01	17.20	208	.00	.02	.00	19.
1.01	4.05	65	.01	.00	.00	1.01	17.25	209	.00	.02	.00	19.
1.01	4.10	66	.01	.00	.00	1.01	17.30	210	.00	.02	.00	19.
1.01	4.15	67	.01	.00	.00	1.01	17.35	211	.00	.02	.00	19.
1.01	4.20	68	.01	.00	.00	1.01	17.40	212	.00	.02	.00	19.
1.01	4.25	69	.01	.00	.00	1.01	17.45	213	.00	.02	.00	19.
1.01	4.30	70	.01	.00	.00	1.01	17.50	214	.00	.02	.00	19.
1.01	4.35	71	.01	.00	.00	1.01	17.55	215	.00	.02	.00	19.
1.01	4.40	72	.01	.00	.00	1.01	18.00	216	.00	.02	.00	19.
1.01	4.45	73	.01	.00	.00	1.01	18.05	217	.00	.02	.00	19.
1.01	4.50	74	.01	.00	.00	1.01	18.10	218	.00	.02	.00	19.
1.01	4.55	75	.01	.00	.00	1.01	18.15	219	.00	.02	.00	19.
1.01	4.60	76	.01	.00	.00	1.01	18.20	220	.00	.02	.00	19.
1.01	4.65	77	.01	.00	.00	1.01	18.25	221	.00	.02	.00	19.
1.01	4.70	78	.01	.00	.00	1.01	18.30	222	.00	.02	.00	19.
1.01	4.75	79	.01	.00	.00	1.01	18.35	223	.00	.02	.00	19.
1.01	4.80	80	.01	.00	.00	1.01	18.40	224	.00	.02	.00	19.
1.01	4.85	81	.01	.00	.00	1.01	18.45	225	.00	.02	.00	19.
1.01	4.90	82	.01	.00	.00	1.01	18.50	226	.00	.02	.00	19.
1.01	4.95	83	.01	.00	.00	1.01	18.55	227	.00	.02	.00	19.
1.01	5.00	84	.01	.00	.00	1.01	19.00	228	.00	.02	.00	19.
1.01	5.05	85	.01	.00	.00	1.01	19.05	229	.00	.02	.00	19.

[illegible]



1.01	11.50	142	.24	.18	.46	.65	1.01	23.50	286	.01	.01	.00	5.
1.01	11.55	143	.24	.18	.45	.63	1.01	23.55	287	.01	.01	.00	5.
1.01	11.60	144	.25	.45	.11	.116	1.02	.00	288	.01	.01	.00	5.
SUM 7.44 5.33 2.11 4096. ( 119.31 134.03 54.31 115.99)													

HYDROGRAPH ROUTING

ROUTE HYDROGRAPH THROUGH DAM

ISTAO	ICOMP	IECON	ITAPE	JPLT	JPRY	INAME	ISTAGL	IAUTO
DAM	1	0	0	0	0	1	0	0
ROUTING DATA								
GROUPS	LOSS	AVG	IFFS	ISAVE	IOPT	IPPP	LSTR	
0.0	0.0	0.0	1	1	0	0	0	
MSIPS	MSIOL	LAG	MSYK	A	TSK	STORE	ISPRAT	
1	0	0	0.00	.000	.000	-684.	-1	
STAGE	444.00	465.00	486.00	487.00	488.00	489.00	490.00	492.00
FLOW	0.0	11.00	24.00	72.00	148.00	278.00	434.00	625.00
SURFACE AREA								
	0.0	0.0	0.0	12.0				692.00
CAPACITIVE								
	0.0	50.0	90.0	102.0				1050.00
ELEVATION								
	444.00	465.00	486.00	487.00	488.00	489.00	490.00	492.00
INFL	SPR	CUW	EXP	ELEV	COOL	CARIA	EXPL	
0.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
DAM DATA								
	INFL	COOL	EXP	PAWIC				
	0.000	0.0	0.0	0.0				
CROSS ELEVATION	10.0	210.0	110.0	430.0	430.0			
AT 25 MILE	10.0	210.0	110.0	430.0	430.0			
ELEVATION	10.0	210.0	110.0	430.0	430.0			

END-OF-FIELD HYDROGRAPH COORDINATES

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 PROJECT 9457: DATE 25 MAR 81 PAGE 9  
 FLOOD HYDROGRAPH PACKAGE - HEC-1  
 PROGRAM M21/C2-1V TIME 13:17:44 CASE

WQREQ	WQREQ	PERIOD	HOURS	INFLOW	OUTFLOW	STORAGE	START
1.01	1.01	1	1.01	0.	0.	50.	064.0
1.01	1.01	2	1.01	0.	0.	50.	064.0
1.01	1.01	3	1.01	0.	0.	50.	064.0
1.01	1.01	4	1.01	0.	0.	50.	064.0
1.01	1.01	5	1.01	0.	0.	50.	064.0
1.01	1.01	6	1.01	0.	0.	50.	064.0
1.01	1.01	7	1.01	0.	0.	50.	064.0
1.01	1.01	8	1.01	0.	0.	50.	064.0
1.01	1.01	9	1.01	0.	0.	50.	064.0
1.01	1.01	10	1.01	0.	0.	50.	064.0
1.01	1.01	11	1.01	0.	0.	50.	064.0
1.01	1.01	12	1.01	0.	0.	50.	064.0
1.01	1.01	13	1.01	0.	0.	50.	064.0
1.01	1.01	14	1.01	0.	0.	50.	064.0
1.01	1.01	15	1.01	0.	0.	50.	064.0
1.01	1.01	16	1.01	0.	0.	50.	064.0
1.01	1.01	17	1.01	0.	0.	50.	064.0
1.01	1.01	18	1.01	0.	0.	50.	064.0
1.01	1.01	19	1.01	0.	0.	50.	064.0
1.01	1.01	20	1.01	0.	0.	50.	064.0
1.01	1.01	21	1.01	0.	0.	50.	064.0
1.01	1.01	22	1.01	0.	0.	50.	064.0
1.01	1.01	23	1.01	0.	0.	50.	064.0
1.01	1.01	24	1.01	0.	0.	50.	064.0
1.01	1.01	25	1.01	0.	0.	50.	064.0
1.01	1.01	26	1.01	0.	0.	50.	064.0
1.01	1.01	27	1.01	0.	0.	50.	064.0
1.01	1.01	28	1.01	0.	0.	50.	064.0
1.01	1.01	29	1.01	0.	0.	50.	064.0
1.01	1.01	30	1.01	0.	0.	50.	064.0
1.01	1.01	31	1.01	0.	0.	50.	064.0
1.01	1.01	32	1.01	0.	0.	50.	064.0
1.01	1.01	33	1.01	0.	0.	50.	064.0
1.01	1.01	34	1.01	0.	0.	50.	064.0
1.01	1.01	35	1.01	0.	0.	50.	064.0
1.01	1.01	36	1.01	0.	0.	50.	064.0
1.01	1.01	37	1.01	0.	0.	50.	064.0
1.01	1.01	38	1.01	0.	0.	50.	064.0
1.01	1.01	39	1.01	0.	0.	50.	064.0
1.01	1.01	40	1.01	0.	0.	50.	064.0
1.01	1.01	41	1.01	0.	0.	50.	064.0
1.01	1.01	42	1.01	0.	0.	50.	064.0
1.01	1.01	43	1.01	0.	0.	50.	064.0
1.01	1.01	44	1.01	0.	0.	50.	064.0
1.01	1.01	45	1.01	0.	0.	50.	064.0
1.01	1.01	46	1.01	0.	0.	50.	064.0
1.01	1.01	47	1.01	0.	0.	50.	064.0
1.01	1.01	48	1.01	0.	0.	50.	064.0
1.01	1.01	49	1.01	0.	0.	50.	064.0
1.01	1.01	50	1.01	0.	0.	50.	064.0
1.01	1.01	51	1.01	0.	0.	50.	064.0
1.01	1.01	52	1.01	0.	0.	50.	064.0
1.01	1.01	53	1.01	0.	0.	50.	064.0
1.01	1.01	54	1.01	0.	0.	50.	064.0

3 L A C V L A T C M  
 PROJECT 9457: DATE 25 MAR 81 PAGE 9  
 FLOOD HYDROGRAPH PACKAGE - HEC-1  
 PROGRAM M21/C2-1V TIME 13:17:44 CASE

1.01	4.35	51	4.38	0.	0.	50.	064.0
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1.01	4.18	51	4.25	0.	50.	064.0
1.01	4.22	52	4.33	0.	50.	064.0
1.01	4.25	53	4.42	0.	50.	064.0
1.01	4.30	54	4.50	0.	50.	064.0
1.01	4.35	55	4.58	0.	50.	064.0
1.01	4.40	56	4.67	0.	50.	064.0
1.01	4.45	57	4.75	0.	50.	064.0
1.01	4.50	58	4.83	0.	50.	064.0
1.01	4.55	59	4.92	0.	50.	064.0
1.01	4.60	60	5.00	0.	50.	064.0
1.01	4.65	61	5.08	0.	50.	064.0
1.01	4.70	62	5.17	0.	50.	064.0
1.01	4.75	63	5.25	0.	50.	064.0
1.01	4.80	64	5.33	0.	50.	064.0
1.01	4.85	65	5.42	0.	50.	064.0
1.01	4.90	66	5.50	0.	50.	064.0
1.01	4.95	67	5.58	0.	50.	064.0
1.01	5.00	68	5.67	0.	50.	064.0
1.01	5.05	69	5.75	0.	50.	064.0
1.01	5.10	70	5.83	0.	50.	064.0
1.01	5.15	71	5.92	0.	50.	064.0
1.01	5.20	72	6.00	0.	50.	064.0
1.01	5.25	73	6.08	0.	50.	064.0
1.01	5.30	74	6.17	0.	50.	064.0
1.01	5.35	75	6.25	0.	50.	064.0
1.01	5.40	76	6.33	0.	50.	064.0
1.01	5.45	77	6.42	0.	50.	064.0
1.01	5.50	78	6.50	0.	50.	064.0
1.01	5.55	79	6.58	0.	50.	064.0
1.01	5.60	80	6.67	0.	50.	064.0
1.01	5.65	81	6.75	0.	50.	064.0
1.01	5.70	82	6.83	0.	50.	064.0
1.01	5.75	83	6.92	0.	50.	064.0
1.01	5.80	84	7.00	0.	50.	064.0
1.01	5.85	85	7.08	0.	50.	064.0
1.01	5.90	86	7.17	0.	50.	064.0
1.01	5.95	87	7.25	0.	50.	064.0
1.01	6.00	88	7.33	0.	50.	064.0
1.01	6.05	89	7.42	0.	50.	064.0
1.01	6.10	90	7.50	0.	50.	064.0
1.01	6.15	91	7.58	0.	50.	064.0
1.01	6.20	92	7.67	0.	50.	064.0
1.01	6.25	93	7.75	0.	50.	064.0
1.01	6.30	94	7.83	0.	50.	064.0
1.01	6.35	95	7.92	0.	50.	064.0
1.01	6.40	96	8.00	0.	50.	064.0
1.01	6.45	97	8.08	0.	50.	064.0
1.01	6.50	98	8.17	0.	50.	064.0
1.01	6.55	99	8.25	0.	50.	064.0
1.01	6.60	100	8.33	0.	50.	064.0
1.01	6.65	101	8.42	0.	50.	064.0
1.01	6.70	102	8.50	0.	50.	064.0
1.01	6.75	103	8.58	0.	50.	064.0
1.01	6.80	104	8.67	0.	50.	064.0
1.01	6.85	105	8.75	0.	50.	064.0
1.01	6.90	106	8.83	0.	50.	064.0
1.01	6.95	107	8.92	0.	50.	064.0
1.01	7.00	108	9.00	0.	50.	064.0
1.01	7.05	109	9.08	0.	50.	064.0
1.01	7.10	110	9.17	0.	50.	064.0
1.01	7.15	111	9.25	0.	50.	064.0
1.01	7.20	112	9.33	0.	50.	064.0
1.01	7.25	113	9.42	0.	50.	064.0
1.01	7.30	114	9.50	0.	50.	064.0
1.01	7.35	115	9.58	0.	50.	064.0
1.01	7.40	116	9.67	0.	50.	064.0
1.01	7.45	117	9.75	0.	50.	064.0
1.01	7.50	118	9.83	0.	50.	064.0
1.01	7.55	119	9.92	0.	50.	064.0
1.01	7.60	120	10.00	0.	50.	064.0
1.01	7.65	121	10.08	0.	50.	064.0
1.01	7.70	122	10.17	0.	50.	064.0
1.01	7.75	123	10.25	0.	50.	064.0
1.01	7.80	124	10.33	0.	50.	064.0
1.01	7.85	125	10.42	0.	50.	064.0
1.01	7.90	126	10.50	0.	50.	064.0
1.01	7.95	127	10.58	0.	50.	064.0
1.01	8.00	128	10.67	0.	50.	064.0
1.01	8.05	129	10.75	0.	50.	064.0
1.01	8.10	130	10.83	0.	50.	064.0
1.01	8.15	131	10.92	0.	50.	064.0
1.01	8.20	132	11.00	0.	50.	064.0
1.01	8.25	133	11.08	0.	50.	064.0
1.01	8.30	134	11.17	0.	50.	064.0
1.01	8.35	135	11.25	0.	50.	064.0
1.01	8.40	136	11.33	0.	50.	064.0
1.01	8.45	137	11.42	0.	50.	064.0
1.01	8.50	138	11.50	0.	50.	064.0
1.01	8.55	139	11.58	0.	50.	064.0
1.01	8.60	140	11.67	0.	50.	064.0
1.01	8.65	141	11.75	0.	50.	064.0
1.01	8.70	142	11.83	0.	50.	064.0
1.01	8.75	143	11.92	0.	50.	064.0
1.01	8.80	144	12.00	0.	50.	064.0
1.01	8.85	145	12.08	0.	50.	064.0
1.01	8.90	146	12.17	0.	50.	064.0
1.01	8.95	147	12.25	0.	50.	064.0
1.01	9.00	148	12.33	0.	50.	064.0
1.01	9.05	149	12.42	0.	50.	064.0
1.01	9.10	150	12.50	0.	50.	064.0
1.01	9.15	151	12.58	0.	50.	064.0
1.01	9.20	152	12.67	0.	50.	064.0
1.01	9.25	153	12.75	0.	50.	064.0
1.01	9.30	154	12.83	0.	50.	064.0
1.01	9.35	155	12.92	0.	50.	064.0
1.01	9.40	156	13.00	0.	50.	064.0
1.01	9.45	157	13.08	0.	50.	064.0
1.01	9.50	158	13.17	0.	50.	064.0
1.01	9.55	159	13.25	0.	50.	064.0
1.01	9.60	160	13.33	0.	50.	064.0
1.01	9.65	161	13.42	0.	50.	064.0
1.01	9.70	162	13.50	0.	50.	064.0
1.01	9.75	163	13.58	0.	50.	064.0
1.01	9.80	164	13.67	0.	50.	064.0
1.01	9.85	165	13.75	0.	50.	064.0
1.01	9.90	166	13.83	0.	50.	064.0
1.01	9.95	167	13.92	0.	50.	064.0
1.01	10.00	168	14.00	0.	50.	064.0
1.01	10.05	169	14.08	0.	50.	064.0
1.01	10.10	170	14.17	0.	50.	064.0
1.01	10.15	171	14.25	0.	50.	064.0
1.01	10.20	172	14.33	0.	50.	064.0
1.01	10.25	173	14.42	0.	50.	064.0
1.01	10.30	174	14.50	0.	50.	064.0
1.01	10.35	175	14.58	0.	50.	064.0
1.01	10.40	176	14.67	0.	50.	064.0
1.01	10.45	177	14.75	0.	50.	064.0
1.01	10.50	178	14.83	0.	50.	064.0
1.01	10.55	179	14.92	0.	50.	064.0
1.01	10.60	180	15.00	0.	50.	064.0
1.01	10.65	181	15.08	0.	50.	064.0
1.01	10.70	182	15.17	0.	50.	064.0
1.01	10.75	183	15.25	0.	50.	064.0
1.01	10.80	184	15.33	0.	50.	064.0
1.01	10.85	185	15.42	0.	50.	064.0
1.01	10.90	186	15.50	0.	50.	064.0
1.01	10.95	187	15.58	0.	50.	064.0
1.01	11.00	188	15.67	0.	50.	064.0
1.01	11.05	189	15.75	0.	50.	064.0
1.01	11.10	190	15.83	0.	50.	064.0
1.01	11.15	191	15.92	0.	50.	064.0
1.01	11.20	192	16.00	0.	50.	064.0
1.01	11.25	193	16.08	0.	50.	064.0
1.01	11.30	194	16.17	0.	50.	064.0
1.01	11.35	195	16.25	0.	50.	064.0
1.01	11.40	196	16.33	0.	50.	064.0
1.01	11.45	197	16.42	0.	50.	064.0
1.01	11.50	198	16.50	0.	50.	064.0
1.01	11.55	199	16.58	0.	50.	064.0
1.01	11.60	200	16.67	0.	50.	064.0
1.01	11.65	201	16.75	0.	50.	064.0
1.01	11.70	202	16.83	0.	50.	064.0
1.01	11.75	203	16.92	0.	50.	064.0
1.01	11.80	204	17.00	0.	50.	064.0
1.01	11.85	205	17.08	0.	50.	064.0
1.01	11.90	206	17.17	0.	50.	064.0
1.01	11.95	207	17.25	0.	50.	064.0
1.01	12.00	208	17.33	0.	50.	064.0
1.01	12.05	209	17.42	0.	50.	064.0
1.01	12.10	210	17.50	0.	50.	064.0
1.01	12.15	211	17.58	0.	50.	064.0
1.01	12.20	212	17.67	0.	50.	064.0
1.01	12.25	213	17.75	0.	50.	064.0
1.01	12.30	214	17.83	0.	50.	064.0
1.01	12.35	215	17.92	0.	50.	064.0
1.01	12.40	216	18.00	0.	50.	064.0
1.01	12.45	217	18.08	0.	50.	064.0
1.01	12.50	218	18.17	0.	50.	064.0
1.01	12.55	219	18.25	0.	50.	064.0
1.01	12.60	220	18.33	0.	50.	064.0
1.01	12.65	221	18.42	0.	50.	064.0
1.01	12.70	222	18.50	0.	50.	064.0
1.01	12.75	223	18.58	0.	50.	064.0
1.01	12.80	224	18.67	0.	50.	064.0
1.01	12.85	225	18.75	0.	50.	064.0
1.01	12.90	226	18.83	0.	50.	064.0
1.01	12.95	227	18.92	0.	50.	064.0
1.01	13.00	228	19.00	0.	50.	064.0
1.01	13.05	229	19.08	0.	50.	064.0
1.01	13.10	230	19.17	0.	50.	064.0
1.01	13.15	231	19.25	0.	50.	064.0
1.01	13.20	232	19.33	0.	50.	064.0
1.01	13.25	233	19.42	0.	50.	064.0
1.01	13.30	234	19.50	0.	50.	064.0
1.01	13.35	235	19.58	0.	50.	064.0
1.01	13.40	236	19.67	0.	50.	064.0
1.01	13.45	237	19.75	0.	50.	064.0
1.01	13.50	238	19.83	0.	50.	064.0
1.01	13.55	239	19.92	0.	50.	064.0
1.01	13.60	240	20.00	0.	50.	064.0
1.01	13.65	241	20.08	0.	50.	064.0
1.01	13.70	242	20.17	0.	50.	064.0
1.01	13.75	243	20.25	0.	50.	064.0
1.01	13.80	244	20.33	0.	50.	064.0
1.01	13.85	245	20.42	0.	50.	064.0
1.01	13.90</					

1.01	9.15	119	9.23	4.	1.	51.	6P4.1
1.01	9.22	112	9.31	4.	2.	51.	6P4.1
1.01	9.28	113	9.42	7.	3.	51.	6P4.1
1.01	9.30	114	9.49	7.	2.	51.	6P4.1
1.01	9.36	115	9.58	8.	2.	51.	6P4.1
1.01	9.40	114	9.47	8.	2.	51.	6P4.1
1.01	9.46	117	9.75	8.	2.	51.	6P4.1
1.01	9.50	113	9.83	9.	2.	51.	6P4.1
1.01	9.56	119	9.62	9.	3.	51.	6P4.1
1.01	10.00	124	10.00	9.	3.	51.	6P4.1
1.01	10.05	121	10.00	9.	3.	51.	6P4.1
1.01	10.10	123	10.17	9.	3.	51.	6P4.1
1.01	10.15	123	10.25	10.	3.	51.	6P4.1
1.01	10.20	124	10.37	10.	3.	51.	6P4.1
1.01	10.25	124	10.42	10.	3.	51.	6P4.1
1.01	10.30	124	10.42	10.	3.	51.	6P4.1
1.01	10.35	124	10.42	10.	3.	51.	6P4.1
1.01	10.40	124	10.42	10.	3.	51.	6P4.1
1.01	10.45	124	10.42	10.	3.	51.	6P4.1
1.01	10.50	124	10.42	10.	3.	51.	6P4.1
1.01	10.55	124	10.42	10.	3.	51.	6P4.1
1.01	11.00	124	10.42	10.	3.	51.	6P4.1
1.01	11.05	124	10.42	10.	3.	51.	6P4.1
1.01	11.10	124	10.42	10.	3.	51.	6P4.1
1.01	11.15	124	10.42	10.	3.	51.	6P4.1
1.01	11.20	124	10.42	10.	3.	51.	6P4.1
1.01	11.25	124	10.42	10.	3.	51.	6P4.1
1.01	11.30	124	10.42	10.	3.	51.	6P4.1
1.01	11.35	124	10.42	10.	3.	51.	6P4.1
1.01	11.40	124	10.42	10.	3.	51.	6P4.1
1.01	11.45	124	10.42	10.	3.	51.	6P4.1
1.01	11.50	124	10.42	10.	3.	51.	6P4.1
1.01	11.55	124	10.42	10.	3.	51.	6P4.1
1.01	12.00	124	10.42	10.	3.	51.	6P4.1
1.01	12.05	124	10.42	10.	3.	51.	6P4.1
1.01	12.10	124	10.42	10.	3.	51.	6P4.1
1.01	12.15	124	10.42	10.	3.	51.	6P4.1
1.01	12.20	124	10.42	10.	3.	51.	6P4.1
1.01	12.25	124	10.42	10.	3.	51.	6P4.1
1.01	12.30	124	10.42	10.	3.	51.	6P4.1
1.01	12.35	124	10.42	10.	3.	51.	6P4.1
1.01	12.40	124	10.42	10.	3.	51.	6P4.1
1.01	12.45	124	10.42	10.	3.	51.	6P4.1
1.01	12.50	124	10.42	10.	3.	51.	6P4.1
1.01	12.55	124	10.42	10.	3.	51.	6P4.1
1.01	13.00	124	10.42	10.	3.	51.	6P4.1
1.01	13.05	124	10.42	10.	3.	51.	6P4.1
1.01	13.10	124	10.42	10.	3.	51.	6P4.1
1.01	13.15	124	10.42	10.	3.	51.	6P4.1
1.01	13.20	124	10.42	10.	3.	51.	6P4.1
1.01	13.25	124	10.42	10.	3.	51.	6P4.1
1.01	13.30	124	10.42	10.	3.	51.	6P4.1
1.01	13.35	124	10.42	10.	3.	51.	6P4.1
1.01	13.40	124	10.42	10.	3.	51.	6P4.1
1.01	13.45	124	10.42	10.	3.	51.	6P4.1
1.01	13.50	124	10.42	10.	3.	51.	6P4.1

1.01	13.55	127	13.54	10.	43.	59.	6P5.4
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1.01	13.35	167	13.59	22.	48.	62.	685.3
1.01	13.40	164	13.67	21.	47.	60.	685.4
1.01	13.45	165	13.73	20.	45.	60.	685.4
1.01	13.50	166	13.83	19.	44.	59.	685.4
1.01	13.55	167	13.92	19.	43.	59.	685.4
1.01	14.00	168	14.00	19.	42.	59.	685.3
1.01	14.05	169	14.08	19.	41.	59.	685.3
1.01	14.10	170	14.17	19.	40.	59.	685.3
1.01	14.15	171	14.25	19.	39.	59.	685.3
1.01	14.20	172	14.33	19.	38.	59.	685.3
1.01	14.25	173	14.42	19.	37.	58.	685.2
1.01	14.30	174	14.50	19.	36.	58.	685.2
1.01	14.35	175	14.58	19.	35.	58.	685.2
1.01	14.40	176	14.67	19.	35.	58.	685.2
1.01	14.45	177	14.75	19.	34.	58.	685.2
1.01	14.50	178	14.83	19.	33.	58.	685.2
1.01	14.55	179	14.92	12.	32.	58.	685.1
1.01	15.00	180	15.00	12.	32.	58.	685.1
1.01	15.05	181	15.08	12.	31.	58.	685.1
1.01	15.10	182	15.17	12.	31.	58.	685.1
1.01	15.15	183	15.25	12.	30.	57.	685.1
1.01	15.20	184	15.33	12.	29.	57.	685.1
1.01	15.25	185	15.42	11.	28.	57.	685.0
1.01	15.30	186	15.50	12.	27.	57.	685.0
1.01	15.35	187	15.58	10.	27.	57.	685.0
1.01	15.40	188	15.67	10.	26.	57.	685.0
1.01	15.45	189	15.75	10.	25.	57.	685.0
1.01	15.50	190	15.83	9.	25.	57.	685.0
1.01	15.55	191	15.92	9.	24.	57.	684.9
1.01	16.00	192	16.00	9.	24.	57.	684.9
1.01	16.05	193	16.08	9.	24.	56.	684.9
1.01	16.10	194	16.17	9.	23.	56.	684.9
1.01	16.15	195	16.25	9.	22.	56.	684.9
1.01	16.20	196	16.33	9.	22.	56.	684.9
1.01	16.25	197	16.42	9.	22.	56.	684.9
1.01	16.30	198	16.50	9.	22.	56.	684.9
1.01	16.35	199	16.58	9.	21.	56.	684.9
1.01	16.40	200	16.67	9.	21.	56.	684.8
1.01	16.45	201	16.75	9.	20.	56.	684.8
1.01	16.50	202	16.83	9.	20.	56.	684.8
1.01	16.55	203	16.92	9.	20.	56.	684.8
1.01	17.00	204	17.00	9.	19.	55.	684.8
1.01	17.05	205	17.08	9.	19.	55.	684.8
1.01	17.10	206	17.17	9.	19.	55.	684.8
1.01	17.15	207	17.25	9.	19.	55.	684.8
1.01	17.20	208	17.33	9.	18.	55.	684.7
1.01	17.25	209	17.42	9.	18.	55.	684.7
1.01	17.30	210	17.50	9.	18.	55.	684.7
1.01	17.35	211	17.58	9.	18.	55.	684.7
1.01	17.40	212	17.67	9.	17.	55.	684.7
1.01	17.45	213	17.75	9.	17.	55.	684.7
1.01	17.50	214	17.83	9.	17.	55.	684.7
1.01	17.55	215	17.92	9.	17.	55.	684.7
1.01	18.00	216	18.00	9.	16.	55.	684.7
1.01	18.05	217	18.08	9.	16.	55.	684.7
1.01	18.10	218	18.17	9.	16.	55.	684.7
1.01	18.15	219	18.25	4.	15.	55.	684.7
1.01	18.20	220	18.33	4.	15.	55.	684.6
1.01	18.25	221	18.42	5.	15.	54.	684.6
1.01	18.30	222	18.50	5.	14.	54.	684.6



1.01	22.55	275	22.02	5.	0.	53.	684.3
1.01	23.05	276	23.02	5.	0.	53.	684.3
1.01	23.55	277	23.08	5.	7.	53.	684.3
1.01	23.57	278	23.17	5.	7.	53.	684.3

PEAK OUTFLOW IS 144. AT TIME 12.58 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	144.	41.	13.	13.	3810.
INCHES	4.	1.	0.	0.	109.
AC-FT	91.07	3.78	4.95	4.95	125.68
THOUS CU 4	20.	20.	26.	26.	33.

RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
AREA IN SQUARE KILOMETERS (KILOMETERS)

	PEAK	6-HOUR	24-HOUR	72-HOUR	AREA
HYDROGRAPH AT	144.	41.	13.	13.	.10
ROUTED TO	144.	41.	13.	13.	.26

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION STORAGE OUTFLOW	INITIAL VALUE 094.00 50. 0.	SPILLWAY CREST 086.00 50. 0.	TOP OF DAM 095.50 60. 49.	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
RATIO P1 P-1 100-year	MAXIMUM W.S. FLEV 095.77	MAXIMUM DEPTH OVER DAM 0.27	MAXIMUM OUTFLOW CFS 144.	MAXIMUM STORAGE AC-FT 62.	1.17	12.58	0.00

2814

M21702-IV









LOSS DATA  
LUMPY STPRN DUTR RTIOL ERAIN STRES RTIOL STNLT CNSTL ALSMR RTIOL  
1 .00 .00 1.00 .00 1.00 -1.00 -82.00 .00  
2 .01 .01 .01 .01 .01 .01 .01 .01  
3 .01 .01 .01 .01 .01 .01 .01 .01  
4 .01 .01 .01 .01 .01 .01 .01 .01  
5 .01 .01 .01 .01 .01 .01 .01 .01  
6 .01 .01 .01 .01 .01 .01 .01 .01  
7 .01 .01 .01 .01 .01 .01 .01 .01  
8 .01 .01 .01 .01 .01 .01 .01 .01  
9 .01 .01 .01 .01 .01 .01 .01 .01  
10 .01 .01 .01 .01 .01 .01 .01 .01  
11 .01 .01 .01 .01 .01 .01 .01 .01  
12 .01 .01 .01 .01 .01 .01 .01 .01  
13 .01 .01 .01 .01 .01 .01 .01 .01  
14 .01 .01 .01 .01 .01 .01 .01 .01  
15 .01 .01 .01 .01 .01 .01 .01 .01  
16 .01 .01 .01 .01 .01 .01 .01 .01  
17 .01 .01 .01 .01 .01 .01 .01 .01  
18 .01 .01 .01 .01 .01 .01 .01 .01  
19 .01 .01 .01 .01 .01 .01 .01 .01  
20 .01 .01 .01 .01 .01 .01 .01 .01  
21 .01 .01 .01 .01 .01 .01 .01 .01  
22 .01 .01 .01 .01 .01 .01 .01 .01  
23 .01 .01 .01 .01 .01 .01 .01 .01  
24 .01 .01 .01 .01 .01 .01 .01 .01  
25 .01 .01 .01 .01 .01 .01 .01 .01  
26 .01 .01 .01 .01 .01 .01 .01 .01  
27 .01 .01 .01 .01 .01 .01 .01 .01  
28 .01 .01 .01 .01 .01 .01 .01 .01  
29 .01 .01 .01 .01 .01 .01 .01 .01

CURVE NO = 0.00 FITNESS = -1.00 EFFECT CM = P2.00

UNIT HYDROGRAPH DATA  
TC = .00 LAG = .20

RECESSION DATA  
STRT = .00 WCEM = .00 RTIOL = 1.00

UNIT HYDROGRAPH 14 END OF PERIOD ORIGINATES, TC = .00 HOURS, LAG = 17.20 VOL = 1.00 S.  
40. 100. 150. 200. 250. 300. 350. 400. 450. 500. 550. 600. 650. 700. 750. 800. 850. 900. 950. 1000.

HR-MIN	PERIOD	RAIN	EXCS	LOSS	END-OF-PERIOD FLOW	COMP Q	PO-DA	HR-MIN	PERIOD	RAIN	EXCS	LOSS	CONF Q
1.01	1.05	1	.00	.00	0.0	0.0	1.01	12.05	165	.59	.45	.14	107.
1.01	1.10	2	.00	.00	0.0	0.0	1.01	12.10	146	.24	.21	.05	102.
1.01	1.15	3	.00	.00	0.0	0.0	1.01	12.15	147	.15	.12	.03	105.
1.01	1.20	4	.00	.00	0.0	0.0	1.01	12.20	146	.08	.06	.01	107.
1.01	1.25	5	.00	.00	0.0	0.0	1.01	12.25	149	.08	.06	.01	129.
1.01	1.30	6	.00	.00	0.0	0.0	1.01	12.30	150	.08	.06	.01	57.
1.01	1.35	7	.00	.00	0.0	0.0	1.01	12.35	151	.04	.05	.01	76.
1.01	1.40	8	.00	.00	0.0	0.0	1.01	12.40	152	.06	.05	.01	62.
1.01	1.45	9	.00	.00	0.0	0.0	1.01	12.45	153	.04	.05	.01	53.
1.01	1.50	10	.00	.00	0.0	0.0	1.01	12.50	154	.06	.05	.01	47.
1.01	1.55	11	.00	.00	0.0	0.0	1.01	12.55	155	.06	.05	.01	43.
1.01	1.00	12	.00	.00	0.0	0.0	1.01	13.00	156	.06	.05	.01	43.
1.01	1.05	13	.00	.00	0.0	0.0	1.01	13.05	157	.03	.02	.00	39.
1.01	1.10	14	.00	.00	0.0	0.0	1.01	13.10	158	.04	.04	.00	34.
1.01	1.15	15	.00	.00	0.0	0.0	1.01	13.15	159	.03	.02	.00	29.
1.01	1.20	16	.00	.00	0.0	0.0	1.01	13.20	160	.03	.02	.00	24.
1.01	1.25	17	.00	.00	0.0	0.0	1.01	13.25	161	.07	.02	.00	22.
1.01	1.30	18	.00	.00	0.0	0.0	1.01	13.30	162	.07	.02	.00	21.
1.01	1.35	19	.00	.00	0.0	0.0	1.01	13.35	163	.01	.01	.00	20.
1.01	1.40	20	.00	.00	0.0	0.0	1.01	13.40	164	.01	.01	.00	17.
1.01	1.45	21	.00	.00	0.0	0.0	1.01	13.45	165	.01	.01	.00	15.
1.01	1.50	22	.00	.00	0.0	0.0	1.01	13.50	166	.01	.01	.00	13.
1.01	1.55	23	.00	.00	0.0	0.0	1.01	13.55	167	.01	.01	.00	11.
1.01	2.00	24	.00	.00	0.0	0.0	1.01	14.00	168	.01	.01	.00	11.
1.01	2.05	25	.00	.00	0.0	0.0	1.01	14.05	169	.01	.01	.00	10.
1.01	2.10	26	.00	.00	0.0	0.0	1.01	14.10	170	.01	.01	.00	10.
1.01	2.15	27	.00	.00	0.0	0.0	1.01	14.15	171	.01	.01	.00	10.
1.01	2.20	28	.00	.00	0.0	0.0	1.01	14.20	172	.01	.01	.00	10.
1.01	2.25	29	.00	.00	0.0	0.0	1.01	14.25	173	.01	.01	.00	10.

1.01	2.375	70	1.01	14.30	174	.01	.01	.00	10.
1.01	2.375	31	1.01	14.35	175	.01	.01	.00	10.
1.01	2.40	34	1.01	14.40	176	.01	.01	.00	10.
1.01	2.45	37	1.01	14.45	177	.01	.01	.00	10.
1.01	2.50	44	1.01	14.50	178	.01	.01	.00	10.
1.01	2.55	53	1.01	14.55	179	.01	.01	.00	10.
1.01	2.60	66	1.01	15.00	180	.01	.01	.00	10.
1.01	2.65	77	1.01	15.05	181	.01	.01	.00	10.
1.01	2.70	86	1.01	15.10	182	.01	.01	.00	9.
1.01	2.75	99	1.01	15.15	183	.01	.01	.00	9.
1.01	2.80	49	1.01	15.20	184	.01	.01	.00	8.
1.01	2.85	41	1.01	15.25	185	.01	.01	.00	8.
1.01	2.90	42	1.01	15.30	186	.01	.01	.00	8.
1.01	2.95	43	1.01	15.35	187	.01	.01	.00	8.
1.01	3.00	44	1.01	15.40	188	.01	.01	.00	7.
1.01	3.05	45	1.01	15.45	189	.01	.01	.00	7.
1.01	3.10	46	1.01	15.50	190	.01	.01	.00	7.
1.01	3.15	47	1.01	15.55	191	.01	.01	.00	7.
1.01	3.20	47	1.01	16.00	192	.01	.01	.00	7.
1.01	3.25	49	1.01	16.05	193	.01	.01	.00	7.
1.01	3.30	51	1.01	16.10	194	.01	.01	.00	7.
1.01	3.35	51	1.01	16.15	195	.01	.01	.00	7.
1.01	3.40	52	1.01	16.20	196	.01	.01	.00	7.
1.01	3.45	53	1.01	16.25	197	.01	.01	.00	7.
1.01	3.50	54	1.01	16.30	198	.01	.01	.00	7.
1.01	3.55	55	1.01	16.35	199	.01	.01	.00	7.
1.01	3.60	56	1.01	16.40	200	.01	.01	.00	7.
1.01	3.65	57	1.01	16.45	201	.01	.01	.00	7.
1.01	3.70	57	1.01	16.50	202	.01	.01	.00	7.
1.01	3.75	58	1.01	16.55	203	.01	.01	.00	7.
1.01	3.80	61	1.01	17.00	204	.01	.01	.00	7.
1.01	3.85	61	1.01	17.05	205	.01	.01	.00	7.
1.01	3.90	62	1.01	17.10	206	.01	.01	.00	7.
1.01	3.95	63	1.01	17.15	207	.01	.01	.00	7.
1.01	4.00	64	1.01	17.20	208	.01	.01	.00	7.
1.01	4.05	65	1.01	17.25	209	.01	.01	.00	7.
1.01	4.10	66	1.01	17.30	210	.01	.01	.00	7.
1.01	4.15	67	1.01	17.35	211	.01	.01	.00	7.
1.01	4.20	68	1.01	17.40	212	.01	.01	.00	7.
1.01	4.25	69	1.01	17.45	213	.01	.01	.00	7.
1.01	4.30	70	1.01	17.50	214	.01	.01	.00	7.
1.01	4.35	71	1.01	17.55	215	.01	.01	.00	7.
1.01	4.40	72	1.01	18.00	216	.01	.01	.00	7.
1.01	4.45	73	1.01	18.05	217	.01	.01	.00	7.
1.01	4.50	74	1.01	18.10	218	.01	.01	.00	6.
1.01	4.55	75	1.01	18.15	219	.01	.01	.00	5.
1.01	4.60	76	1.01	18.20	220	.01	.01	.00	4.
1.01	4.65	77	1.01	18.25	221	.01	.01	.00	4.
1.01	4.70	77	1.01	18.30	222	.01	.01	.00	4.
1.01	4.75	78	1.01	18.35	223	.01	.01	.00	4.
1.01	4.80	80	1.01	18.40	224	.01	.01	.00	4.
1.01	4.85	81	1.01	18.45	225	.01	.01	.00	3.
1.01	4.90	81	1.01	18.50	226	.01	.01	.00	3.
1.01	4.95	82	1.01	18.55	227	.01	.01	.00	3.
1.01	5.00	84	1.01	19.00	228	.01	.01	.00	3.
1.01	5.05	84	1.01	19.05	229	.01	.01	.00	3.

1.01	7.10	87	.01	.00	.01	1.01	19.10	270	.00	.00	.00	3.
1.01	7.15	87	.01	.00	.01	1.01	19.15	271	.00	.00	.00	3.
1.01	7.20	87	.01	.00	.01	1.01	19.20	272	.00	.00	.00	3.
1.01	7.25	87	.01	.00	.01	1.01	19.25	273	.00	.00	.00	3.
1.01	7.30	87	.01	.00	.01	1.01	19.30	274	.00	.00	.00	3.
1.01	7.35	87	.01	.00	.01	1.01	19.35	275	.00	.00	.00	3.
1.01	7.40	87	.01	.00	.01	1.01	19.40	276	.00	.00	.00	3.
1.01	7.45	87	.01	.00	.01	1.01	19.45	277	.00	.00	.00	3.
1.01	7.50	87	.01	.00	.01	1.01	19.50	278	.00	.00	.00	3.
1.01	7.55	87	.01	.00	.01	1.01	19.55	279	.00	.00	.00	3.
1.01	8.00	87	.01	.00	.01	1.01	20.00	280	.00	.00	.00	3.
1.01	8.05	87	.01	.00	.01	1.01	20.05	281	.00	.00	.00	3.
1.01	8.10	87	.01	.00	.01	1.01	20.10	282	.00	.00	.00	3.
1.01	8.15	87	.01	.00	.01	1.01	20.15	283	.00	.00	.00	3.
1.01	8.20	87	.01	.00	.01	1.01	20.20	284	.00	.00	.00	3.
1.01	8.25	87	.01	.00	.01	1.01	20.25	285	.00	.00	.00	3.
1.01	8.30	87	.01	.00	.01	1.01	20.30	286	.00	.00	.00	3.
1.01	8.35	87	.01	.00	.01	1.01	20.35	287	.00	.00	.00	3.
1.01	8.40	87	.01	.00	.01	1.01	20.40	288	.00	.00	.00	3.
1.01	8.45	87	.01	.00	.01	1.01	20.45	289	.00	.00	.00	3.
1.01	8.50	87	.01	.00	.01	1.01	20.50	290	.00	.00	.00	3.
1.01	8.55	87	.01	.00	.01	1.01	20.55	291	.00	.00	.00	3.
1.01	9.00	87	.01	.00	.01	1.01	21.00	292	.00	.00	.00	3.
1.01	9.05	87	.01	.00	.01	1.01	21.05	293	.00	.00	.00	3.
1.01	9.10	87	.01	.00	.01	1.01	21.10	294	.00	.00	.00	3.
1.01	9.15	87	.01	.00	.01	1.01	21.15	295	.00	.00	.00	3.
1.01	9.20	87	.01	.00	.01	1.01	21.20	296	.00	.00	.00	3.
1.01	9.25	87	.01	.00	.01	1.01	21.25	297	.00	.00	.00	3.
1.01	9.30	87	.01	.00	.01	1.01	21.30	298	.00	.00	.00	3.
1.01	9.35	87	.01	.00	.01	1.01	21.35	299	.00	.00	.00	3.
1.01	9.40	87	.01	.00	.01	1.01	21.40	300	.00	.00	.00	3.
1.01	9.45	87	.01	.00	.01	1.01	21.45	301	.00	.00	.00	3.
1.01	9.50	87	.01	.00	.01	1.01	21.50	302	.00	.00	.00	3.
1.01	9.55	87	.01	.00	.01	1.01	21.55	303	.00	.00	.00	3.
1.01	10.00	87	.01	.00	.01	1.01	22.00	304	.00	.00	.00	3.
1.01	10.05	87	.01	.00	.01	1.01	22.05	305	.00	.00	.00	3.
1.01	10.10	87	.01	.00	.01	1.01	22.10	306	.00	.00	.00	3.
1.01	10.15	87	.01	.00	.01	1.01	22.15	307	.00	.00	.00	3.
1.01	10.20	87	.01	.00	.01	1.01	22.20	308	.00	.00	.00	3.
1.01	10.25	87	.01	.00	.01	1.01	22.25	309	.00	.00	.00	3.
1.01	10.30	87	.01	.00	.01	1.01	22.30	310	.00	.00	.00	3.
1.01	10.35	87	.01	.00	.01	1.01	22.35	311	.00	.00	.00	3.
1.01	10.40	87	.01	.00	.01	1.01	22.40	312	.00	.00	.00	3.
1.01	10.45	87	.01	.00	.01	1.01	22.45	313	.00	.00	.00	3.
1.01	10.50	87	.01	.00	.01	1.01	22.50	314	.00	.00	.00	3.
1.01	10.55	87	.01	.00	.01	1.01	22.55	315	.00	.00	.00	3.
1.01	11.00	87	.01	.00	.01	1.01	23.00	316	.00	.00	.00	3.
1.01	11.05	87	.01	.00	.01	1.01	23.05	317	.00	.00	.00	3.
1.01	11.10	87	.01	.00	.01	1.01	23.10	318	.00	.00	.00	3.
1.01	11.15	87	.01	.00	.01	1.01	23.15	319	.00	.00	.00	3.
1.01	11.20	87	.01	.00	.01	1.01	23.20	320	.00	.00	.00	3.
1.01	11.25	87	.01	.00	.01	1.01	23.25	321	.00	.00	.00	3.
1.01	11.30	87	.01	.00	.01	1.01	23.30	322	.00	.00	.00	3.
1.01	11.35	87	.01	.00	.01	1.01	23.35	323	.00	.00	.00	3.
1.01	11.40	87	.01	.00	.01	1.01	23.40	324	.00	.00	.00	3.
1.01	11.45	87	.01	.00	.01	1.01	23.45	325	.00	.00	.00	3.

1.01	11.50	142	.15	.09	.06	35.	1.01	23.50	286	.00	.00	.00	3.
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1001 11.45 141 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1002 11.45 142 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1003 11.45 143 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1004 11.45 144 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1005 11.45 145 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1006 11.45 146 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1007 11.45 147 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1008 11.45 148 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1009 11.45 149 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.  
 1010 11.45 150 .00 .00 .00 1.01 23.45 295 .00 .00 .00 3.

BLACK R V E A T C H  
 PROJECT 9457: DATE 26 MAR 81 PAGE 7  
 FLOOD HYDROGRAPH PACKAGE - MLC-1  
 PROGRAM N21/P2-1V TIME 08:40:00 CASE

1001 11.50 142 .15 .09 .06 1.01 23.50 296 .00 .00 .00 3.  
 1002 11.55 143 .15 .09 .06 1.01 23.55 297 .00 .00 .00 3.  
 1003 12.00 144 .37 .26 .11 1.02 .00 298 .00 .00 .00 3.  
 SUM 5.21 3.27 1.54 24P1.  
 ( 172.31 83.31 45.31 70.25)

PEAK 0-HOUR 24-HOUR 72-HOUR TOTAL VOLUME  
 CFS 185. 21. 9. 242C.  
 CFS 5. 1. 0. 71.  
 INCHES 2.68 3.26 3.26 3.26  
 MP 82.40 82.30 82.30 82.30  
 AC-17 16. 17. 17. 17.  
 THOUS CU YD 12. 21. 21. 21.

# HYDROGRAPH ROUTING

## ROUTE HYDROGRAPH THROUGH DAM

ISTAR ICOMP IFCM IFAPE JPLE JPRY IMAPE ISTAR IAUO  
 LAM 1 1 1 1 1 1 1 1 1 1  
 ROUTING DATA  
 CLOS CLOS AVG IUES IFAPE IOTY IPPP LSTR  
 1 1 1 1 1 1 1 1 1 1  
 NSIES NSTEL LAG APSR X TSN STORA ISPRAT  
 1 1 1 1 1 1 1 1 1 1

STAGE 234.50 244.50 254.50 264.50 274.50 284.50 294.50 304.50 314.50 324.50  
 FLOW 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00  
 SURFACE AREA 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

## CAPACITY

CAPACITY 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

## ELEVATION

ELEVATION 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

CURST ELEVATION 150. 210. 300. 400. 500. 600. 700. 800. 900. 1000.  
 FT UP ELEV. 150. 210. 300. 400. 500. 600. 700. 800. 900. 1000.  
 ELEVATION 150. 210. 300. 400. 500. 600. 700. 800. 900. 1000.

END OF PLEFING HYDROGRAPH COORDINATES

MC-DA	HE-MN	PERIOD	HOURS	INFLO	OUTFLO	STORAGE	STAGE
1.01	0.5	1	0.0	0.	0.	50.	694.0
1.01	0.5	2	0.17	0.	0.	50.	694.0
1.01	0.5	3	0.33	0.	0.	50.	694.0
1.01	0.5	4	0.50	0.	0.	50.	694.0
1.01	0.5	5	0.67	0.	0.	50.	694.0
1.01	0.5	6	0.83	0.	0.	50.	694.0
1.01	0.5	7	1.00	0.	0.	50.	694.0
1.01	0.5	8	1.17	0.	0.	50.	694.0
1.01	0.5	9	1.33	0.	0.	50.	694.0
1.01	0.5	10	1.50	0.	0.	50.	694.0
1.01	0.5	11	1.67	0.	0.	50.	694.0
1.01	0.5	12	1.83	0.	0.	50.	694.0
1.01	0.5	13	2.00	0.	0.	50.	694.0
1.01	0.5	14	2.17	0.	0.	50.	694.0
1.01	0.5	15	2.33	0.	0.	50.	694.0
1.01	0.5	16	2.50	0.	0.	50.	694.0
1.01	0.5	17	2.67	0.	0.	50.	694.0
1.01	0.5	18	2.83	0.	0.	50.	694.0
1.01	0.5	19	3.00	0.	0.	50.	694.0
1.01	0.5	20	3.17	0.	0.	50.	694.0
1.01	0.5	21	3.33	0.	0.	50.	694.0
1.01	0.5	22	3.50	0.	0.	50.	694.0
1.01	0.5	23	3.67	0.	0.	50.	694.0
1.01	0.5	24	3.83	0.	0.	50.	694.0
1.01	0.5	25	4.00	0.	0.	50.	694.0
1.01	0.5	26	4.17	0.	0.	50.	694.0
1.01	0.5	27	4.33	0.	0.	50.	694.0
1.01	0.5	28	4.50	0.	0.	50.	694.0
1.01	0.5	29	4.67	0.	0.	50.	694.0
1.01	0.5	30	4.83	0.	0.	50.	694.0
1.01	0.5	31	5.00	0.	0.	50.	694.0
1.01	0.5	32	5.17	0.	0.	50.	694.0
1.01	0.5	33	5.33	0.	0.	50.	694.0
1.01	0.5	34	5.50	0.	0.	50.	694.0
1.01	0.5	35	5.67	0.	0.	50.	694.0
1.01	0.5	36	5.83	0.	0.	50.	694.0
1.01	0.5	37	6.00	0.	0.	50.	694.0
1.01	0.5	38	6.17	0.	0.	50.	694.0
1.01	0.5	39	6.33	0.	0.	50.	694.0
1.01	0.5	40	6.50	0.	0.	50.	694.0
1.01	0.5	41	6.67	0.	0.	50.	694.0
1.01	0.5	42	6.83	0.	0.	50.	694.0
1.01	0.5	43	7.00	0.	0.	50.	694.0
1.01	0.5	44	7.17	0.	0.	50.	694.0
1.01	0.5	45	7.33	0.	0.	50.	694.0
1.01	0.5	46	7.50	0.	0.	50.	694.0
1.01	0.5	47	7.67	0.	0.	50.	694.0
1.01	0.5	48	7.83	0.	0.	50.	694.0
1.01	0.5	49	8.00	0.	0.	50.	694.0
1.01	0.5	50	8.17	0.	0.	50.	694.0
1.01	0.5	51	8.33	0.	0.	50.	694.0
1.01	0.5	52	8.50	0.	0.	50.	694.0
1.01	0.5	53	8.67	0.	0.	50.	694.0
1.01	0.5	54	8.83	0.	0.	50.	694.0
1.01	0.5	55	9.00	0.	0.	50.	694.0



1.01	4.35	55	4.58	0.	0.	50.	0P4.0
1.01	4.40	56	4.57	0.	0.	50.	0P4.0
1.01	4.45	57	4.55	0.	0.	50.	0P4.0
1.01	4.50	58	4.53	0.	0.	50.	0P4.0
1.01	4.55	59	4.52	0.	0.	50.	0P4.0
1.01	4.60	60	4.50	0.	0.	50.	0P4.0
1.01	4.65	61	4.48	0.	0.	50.	0P4.0
1.01	4.70	62	4.47	0.	0.	50.	0P4.0
1.01	4.75	63	4.45	0.	0.	50.	0P4.0
1.01	4.80	64	4.43	0.	0.	50.	0P4.0
1.01	4.85	65	4.42	0.	0.	50.	0P4.0
1.01	4.90	66	4.40	0.	0.	50.	0P4.0
1.01	4.95	67	4.38	0.	0.	50.	0P4.0
1.01	5.00	68	4.37	0.	0.	50.	0P4.0
1.01	5.05	69	4.35	0.	0.	50.	0P4.0
1.01	5.10	70	4.33	0.	0.	50.	0P4.0
1.01	5.15	71	4.32	0.	0.	50.	0P4.0
1.01	5.20	72	4.30	0.	0.	50.	0P4.0
1.01	5.25	73	4.28	0.	0.	50.	0P4.0
1.01	5.30	74	4.27	0.	0.	50.	0P4.0
1.01	5.35	75	4.25	0.	0.	50.	0P4.0
1.01	5.40	76	4.23	0.	0.	50.	0P4.0
1.01	5.45	77	4.22	0.	0.	50.	0P4.0
1.01	5.50	78	4.20	0.	0.	50.	0P4.0
1.01	5.55	79	4.18	0.	0.	50.	0P4.0
1.01	5.60	80	4.17	0.	0.	50.	0P4.0
1.01	5.65	81	4.15	0.	0.	50.	0P4.0
1.01	5.70	82	4.13	0.	0.	50.	0P4.0
1.01	5.75	83	4.12	0.	0.	50.	0P4.0
1.01	5.80	84	4.10	0.	0.	50.	0P4.0
1.01	5.85	85	4.08	0.	0.	50.	0P4.0
1.01	5.90	86	4.07	0.	0.	50.	0P4.0
1.01	5.95	87	4.05	0.	0.	50.	0P4.0
1.01	6.00	88	4.03	0.	0.	50.	0P4.0
1.01	6.05	89	4.02	0.	0.	50.	0P4.0
1.01	6.10	90	4.00	0.	0.	50.	0P4.0
1.01	6.15	91	3.98	0.	0.	50.	0P4.0
1.01	6.20	92	3.97	0.	0.	50.	0P4.0
1.01	6.25	93	3.95	0.	0.	50.	0P4.0
1.01	6.30	94	3.93	0.	0.	50.	0P4.0
1.01	6.35	95	3.92	0.	0.	50.	0P4.0
1.01	6.40	96	3.90	0.	0.	50.	0P4.0
1.01	6.45	97	3.88	0.	0.	50.	0P4.0
1.01	6.50	98	3.87	0.	0.	50.	0P4.0
1.01	6.55	99	3.85	0.	0.	50.	0P4.0
1.01	6.60	100	3.83	0.	0.	50.	0P4.0
1.01	6.65	101	3.82	0.	0.	50.	0P4.0
1.01	6.70	102	3.80	0.	0.	50.	0P4.0
1.01	6.75	103	3.78	0.	0.	50.	0P4.0
1.01	6.80	104	3.77	0.	0.	50.	0P4.0
1.01	6.85	105	3.75	0.	0.	50.	0P4.0
1.01	6.90	106	3.73	0.	0.	50.	0P4.0
1.01	6.95	107	3.72	0.	0.	50.	0P4.0
1.01	7.00	108	3.70	0.	0.	50.	0P4.0
1.01	7.05	109	3.68	0.	0.	50.	0P4.0
1.01	7.10	110	3.67	0.	0.	50.	0P4.0

1.01	9.15	111	9.25	1.	51.	64.0
1.01	9.20	112	9.30	1.	51.	64.0
1.01	9.25	113	9.40	1.	51.	64.0
1.01	9.30	114	9.50	1.	51.	64.0
1.01	9.35	115	9.50	1.	51.	64.0
1.01	9.40	116	9.67	1.	51.	64.0
1.01	9.45	117	9.75	1.	51.	64.0
1.01	9.50	118	9.83	1.	51.	64.0
1.01	9.55	119	9.92	1.	51.	64.0
1.01	10.00	120	10.00	1.	51.	64.0
1.01	10.05	121	10.06	1.	51.	64.0
1.01	10.10	122	10.17	1.	51.	64.0
1.01	10.15	123	10.25	1.	51.	64.0
1.01	10.20	124	10.33	1.	51.	64.0
1.01	10.25	125	10.42	1.	51.	64.0
1.01	10.30	126	10.50	1.	51.	64.0
1.01	10.35	127	10.58	1.	51.	64.0
1.01	10.40	128	10.67	1.	51.	64.0
1.01	10.45	129	10.75	1.	51.	64.0
1.01	10.50	130	10.83	1.	51.	64.0
1.01	10.55	131	10.92	1.	51.	64.0
1.01	11.00	132	11.00	1.	51.	64.0
1.01	11.05	133	11.08	1.	51.	64.0
1.01	11.10	134	11.17	1.	51.	64.0
1.01	11.15	135	11.25	1.	51.	64.0
1.01	11.20	136	11.33	1.	51.	64.0
1.01	11.25	137	11.42	1.	51.	64.0
1.01	11.30	138	11.50	1.	51.	64.0
1.01	11.35	139	11.58	1.	51.	64.0
1.01	11.40	140	11.67	1.	51.	64.0
1.01	11.45	141	11.75	1.	51.	64.0
1.01	11.50	142	11.83	1.	51.	64.0
1.01	11.55	143	11.92	1.	51.	64.0
1.01	12.00	144	12.00	1.	51.	64.0
1.01	12.05	145	12.08	1.	51.	64.0
1.01	12.10	146	12.17	1.	51.	64.0
1.01	12.15	147	12.25	1.	51.	64.0
1.01	12.20	148	12.33	1.	51.	64.0
1.01	12.25	149	12.42	1.	51.	64.0
1.01	12.30	150	12.50	1.	51.	64.0
1.01	12.35	151	12.58	1.	51.	64.0
1.01	12.40	152	12.67	1.	51.	64.0
1.01	12.45	153	12.75	1.	51.	64.0
1.01	12.50	154	12.83	1.	51.	64.0
1.01	12.55	155	12.92	1.	51.	64.0
1.01	13.00	156	13.00	1.	51.	64.0
1.01	13.05	157	13.08	1.	51.	64.0
1.01	13.10	158	13.17	1.	51.	64.0
1.01	13.15	159	13.25	1.	51.	64.0
1.01	13.20	160	13.33	1.	51.	64.0
1.01	13.25	161	13.42	1.	51.	64.0
1.01	13.30	162	13.50	1.	51.	64.0
1.01	13.35	163	13.58	1.	51.	64.0
1.01	13.40	164	13.67	1.	51.	64.0
1.01	13.45	165	13.75	1.	51.	64.0
1.01	13.50	166	13.83	1.	51.	64.0

1.01	13.55	167	13.92	1.	51.	64.0
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1.01 13:30	167	17.92	17.	31.	59.	6P5.1
1.01 13:35	168	17.92	17.	31.	59.	6P5.1
1.01 13:40	169	17.92	17.	31.	59.	6P5.1
1.01 13:45	170	17.92	17.	31.	59.	6P5.1
1.01 13:50	171	17.92	17.	31.	59.	6P5.1
1.01 13:55	172	17.92	17.	31.	59.	6P5.1
1.01 14:00	173	17.92	17.	31.	59.	6P5.1
1.01 14:05	174	17.92	17.	31.	59.	6P5.1
1.01 14:10	175	17.92	17.	31.	59.	6P5.1
1.01 14:15	176	17.92	17.	31.	59.	6P5.1
1.01 14:20	177	17.92	17.	31.	59.	6P5.1
1.01 14:25	178	17.92	17.	31.	59.	6P5.1
1.01 14:30	179	17.92	17.	31.	59.	6P5.1
1.01 14:35	180	17.92	17.	31.	59.	6P5.1
1.01 14:40	181	17.92	17.	31.	59.	6P5.1
1.01 14:45	182	17.92	17.	31.	59.	6P5.1
1.01 14:50	183	17.92	17.	31.	59.	6P5.1
1.01 14:55	184	17.92	17.	31.	59.	6P5.1
1.01 15:00	185	17.92	17.	31.	59.	6P5.1
1.01 15:05	186	17.92	17.	31.	59.	6P5.1
1.01 15:10	187	17.92	17.	31.	59.	6P5.1
1.01 15:15	188	17.92	17.	31.	59.	6P5.1
1.01 15:20	189	17.92	17.	31.	59.	6P5.1
1.01 15:25	190	17.92	17.	31.	59.	6P5.1
1.01 15:30	191	17.92	17.	31.	59.	6P5.1
1.01 15:35	192	17.92	17.	31.	59.	6P5.1
1.01 15:40	193	17.92	17.	31.	59.	6P5.1
1.01 15:45	194	17.92	17.	31.	59.	6P5.1
1.01 15:50	195	17.92	17.	31.	59.	6P5.1
1.01 15:55	196	17.92	17.	31.	59.	6P5.1
1.01 16:00	197	17.92	17.	31.	59.	6P5.1
1.01 16:05	198	17.92	17.	31.	59.	6P5.1
1.01 16:10	199	17.92	17.	31.	59.	6P5.1
1.01 16:15	200	17.92	17.	31.	59.	6P5.1
1.01 16:20	201	17.92	17.	31.	59.	6P5.1
1.01 16:25	202	17.92	17.	31.	59.	6P5.1
1.01 16:30	203	17.92	17.	31.	59.	6P5.1
1.01 16:35	204	17.92	17.	31.	59.	6P5.1
1.01 16:40	205	17.92	17.	31.	59.	6P5.1
1.01 16:45	206	17.92	17.	31.	59.	6P5.1
1.01 16:50	207	17.92	17.	31.	59.	6P5.1
1.01 16:55	208	17.92	17.	31.	59.	6P5.1
1.01 17:00	209	17.92	17.	31.	59.	6P5.1
1.01 17:05	210	17.92	17.	31.	59.	6P5.1
1.01 17:10	211	17.92	17.	31.	59.	6P5.1
1.01 17:15	212	17.92	17.	31.	59.	6P5.1
1.01 17:20	213	17.92	17.	31.	59.	6P5.1
1.01 17:25	214	17.92	17.	31.	59.	6P5.1
1.01 17:30	215	17.92	17.	31.	59.	6P5.1
1.01 17:35	216	17.92	17.	31.	59.	6P5.1
1.01 17:40	217	17.92	17.	31.	59.	6P5.1
1.01 17:45	218	17.92	17.	31.	59.	6P5.1
1.01 17:50	219	17.92	17.	31.	59.	6P5.1
1.01 17:55	220	17.92	17.	31.	59.	6P5.1
1.01 18:00	221	17.92	17.	31.	59.	6P5.1
1.01 18:05	222	17.92	17.	31.	59.	6P5.1
1.01 18:10	223	17.92	17.	31.	59.	6P5.1
1.01 18:15	224	17.92	17.	31.	59.	6P5.1
1.01 18:20	225	17.92	17.	31.	59.	6P5.1
1.01 18:25	226	17.92	17.	31.	59.	6P5.1
1.01 18:30	227	17.92	17.	31.	59.	6P5.1

1.71	16.57	223	17.50	4.	11.	54.	6P4.5
1.01	16.46	224	17.67	4.	11.	54.	6P4.5
1.01	16.45	225	17.75	3.	11.	54.	6P4.5
1.01	16.50	226	18.43	3.	11.	54.	6P4.5
1.01	16.55	227	18.92	3.	10.	53.	6P4.5
1.01	16.50	228	19.00	3.	10.	53.	6P4.5
1.01	16.95	229	19.08	3.	10.	53.	6P4.5
1.01	16.10	230	19.17	3.	10.	53.	6P4.5
1.01	16.15	231	19.25	3.	10.	53.	6P4.5
1.01	16.20	232	19.33	3.	10.	53.	6P4.5
1.01	16.25	233	19.42	3.	9.	52.	6P4.5
1.01	16.30	234	19.50	3.	9.	52.	6P4.5
1.01	16.35	235	19.58	3.	9.	52.	6P4.5
1.01	16.40	236	19.67	3.	9.	52.	6P4.5
1.01	16.45	237	19.75	3.	9.	52.	6P4.5
1.01	16.50	238	19.83	3.	9.	52.	6P4.5
1.01	16.55	239	19.92	3.	9.	52.	6P4.5
1.01	16.58	240	20.00	3.	9.	52.	6P4.5
1.01	16.59	241	20.08	3.	8.	51.	6P4.5
1.01	16.59	242	20.17	3.	8.	51.	6P4.5
1.01	16.59	243	20.25	3.	8.	51.	6P4.5
1.01	16.59	244	20.33	3.	8.	51.	6P4.5
1.01	16.59	245	20.42	3.	8.	51.	6P4.5
1.01	16.59	246	20.50	3.	8.	51.	6P4.5
1.01	16.59	247	20.58	3.	8.	51.	6P4.5
1.01	16.59	248	20.67	3.	8.	51.	6P4.5
1.01	16.59	249	20.75	3.	8.	51.	6P4.5
1.01	16.59	250	20.83	3.	7.	50.	6P4.5
1.01	16.59	251	20.92	3.	7.	50.	6P4.5
1.01	16.59	252	21.00	3.	7.	50.	6P4.5
1.01	16.59	253	21.08	3.	7.	50.	6P4.5
1.01	16.59	254	21.17	3.	7.	50.	6P4.5
1.01	16.59	255	21.25	3.	7.	50.	6P4.5
1.01	16.59	256	21.33	3.	7.	50.	6P4.5
1.01	16.59	257	21.42	3.	7.	50.	6P4.5
1.01	16.59	258	21.50	3.	7.	50.	6P4.5
1.01	16.59	259	21.58	3.	7.	50.	6P4.5
1.01	16.59	260	21.67	3.	7.	50.	6P4.5
1.01	16.59	261	21.75	3.	7.	50.	6P4.5
1.01	16.59	262	21.83	3.	7.	50.	6P4.5
1.01	16.59	263	21.92	3.	7.	50.	6P4.5
1.01	16.59	264	22.00	3.	6.	49.	6P4.5
1.01	16.59	265	22.08	3.	6.	49.	6P4.5
1.01	16.59	266	22.17	3.	6.	49.	6P4.5
1.01	16.59	267	22.25	3.	6.	49.	6P4.5
1.01	16.59	268	22.33	3.	6.	49.	6P4.5
1.01	16.59	269	22.42	3.	6.	49.	6P4.5
1.01	16.59	270	22.50	3.	6.	49.	6P4.5
1.01	16.59	271	22.58	3.	6.	49.	6P4.5
1.01	16.59	272	22.67	3.	6.	49.	6P4.5
1.01	16.59	273	22.75	3.	6.	49.	6P4.5
1.01	16.59	274	22.83	3.	6.	49.	6P4.5
1.01	16.59	275	22.92	3.	6.	49.	6P4.5
1.01	16.59	276	23.00	3.	6.	49.	6P4.5
1.01	16.59	277	23.08	3.	6.	49.	6P4.5
1.01	16.59	278	23.17	3.	6.	49.	6P4.5

1.01 22.55	275	22.02	5.	52.	694.3
1.01 23.09	276	23.09	6.	52.	694.3
1.01 23.25	277	23.09	6.	52.	694.3
1.01 23.10	277	23.17	6.	52.	694.3

1.01 23.15	279	23.25	1.	52.	694.3
1.01 23.20	280	23.33	5.	52.	694.2
1.01 23.25	281	23.42	5.	52.	694.2
1.01 23.30	282	23.50	5.	52.	694.2
1.01 23.35	283	23.58	5.	52.	694.2
1.01 23.40	284	23.67	5.	52.	694.2
1.01 23.45	285	23.75	5.	52.	694.2
1.01 23.50	286	23.83	5.	52.	694.2
1.01 23.55	287	23.92	5.	52.	694.2
1.01 24.00	288	24.00	5.	52.	694.2

PEAK OUTFLOW IS 34. AT TIME 13.48 HOURS

PEAK	0-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	73.	8.	8.	2314.
1000 CFS	1.	0.	0.	66.
10000 CFS	2.17	1.90	2.09	2.09
100000 CFS	55.04	75.97	75.97	75.97
1000000 CFS	12.	16.	16.	16.
10000000 CFS	14.	20.	20.	20.

RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
 AREA IN SQUARE KILOMETERS

HYDROGRAPH AT	HEAD	PEAK	0-HOUR	24-HOUR	72-HOUR	AREA
		185.	29.	9.	9.	.10
		5.253E	.013E	.253E	.253E	.26
ACUTED TC	DA"	35.	23.	8.	8.	.10
		1.073E	.063E	.233E	.233E	.26

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1

ELEVATION STOPALL OUTFLOW	INITIAL VALUE OP%CC 50. 0.	SPILLWAY CREST CP%CO 50. 0.	TOP OF DAM OPS.50 50. 49.	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
MAXIMUM RESERVOIR B.S.ELEV	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS				
10-Year 4.00	59.	38.		00	13.0P	00

SPIN

NETTLB FIN

ATE  
MED  
-8